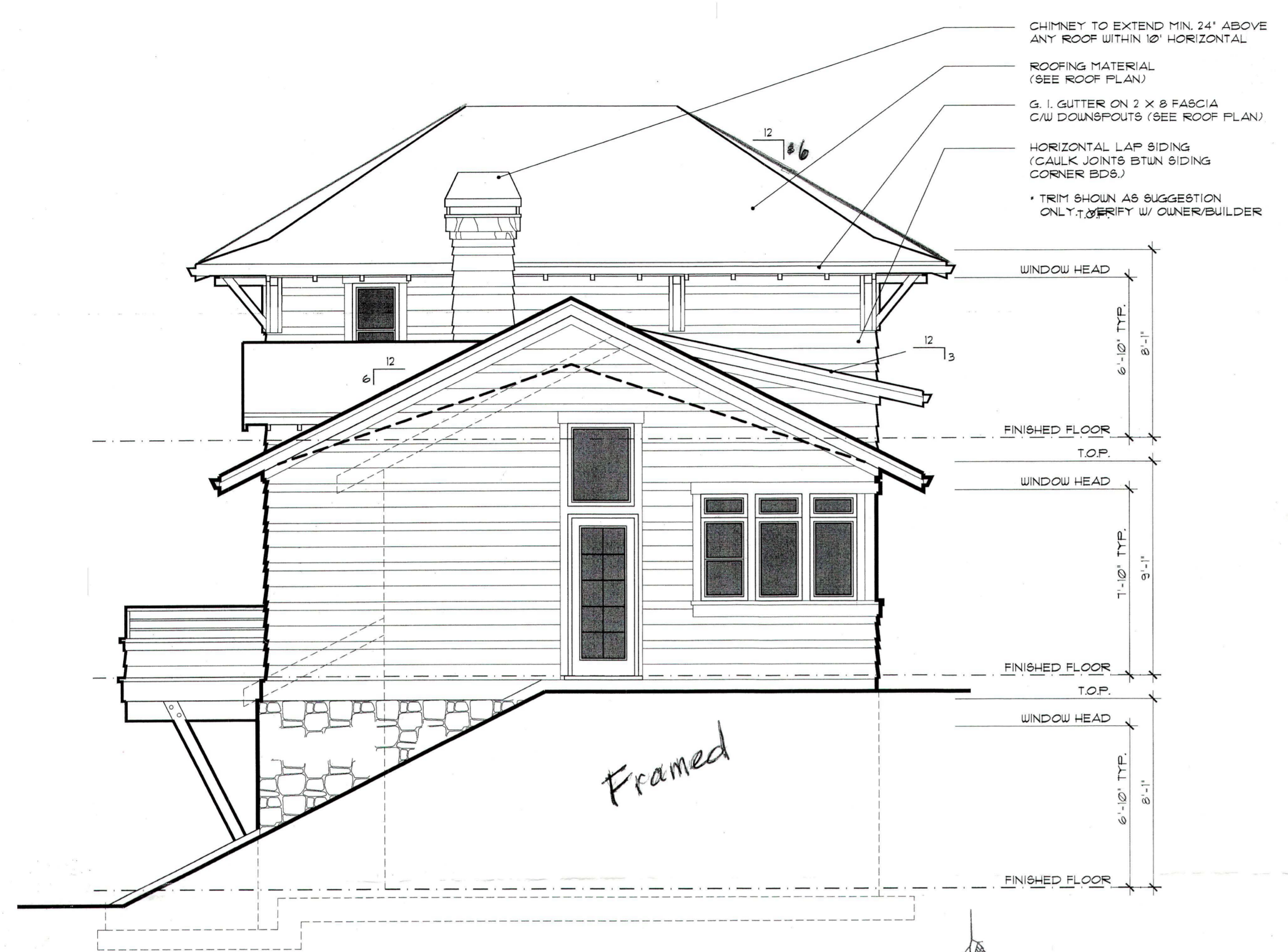


#2008-119
HALLIBURTON

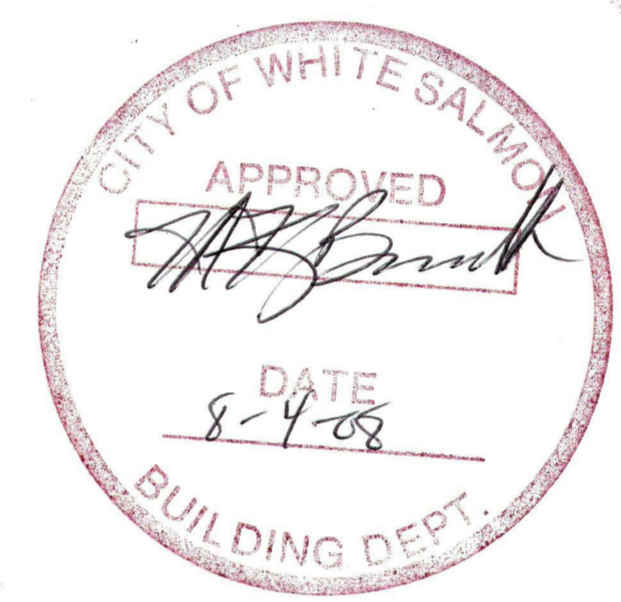


RIGHT ELEVATION
SCALE: 1/4" = 1'-0"

THE CITY OF WHITE SALMON
BUILDING DEPARTMENT

THESE PLANS HAVE BEEN REVIEWED FOR CONFORMANCE WITH STATE BUILDING CODES, AS ADOPTED BY THE CITY OF WHITE SALMON. ANY PROPOSED CHANGES FROM THE REVIEWED PLANS SHALL BE SUBMITTED FOR REVIEW BEFORE CONSTRUCTION. THIS PLAN REVIEW SHALL NOT PREVENT THE CORRECTION OF ERRORS OR VIOLATIONS THAT ARE FOUND TO EXIST IN THE SUBJECT CONSTRUCTION.

[Signature]
Signature



Notice
Approved plans shall be accompanied by the plan review in order to be valid.

SEE PLAN REVIEW NOTES!

APPROVED PLANS AND PERMIT MUST BE ON JOB SITE



Alan Mascord Design Associates, Inc. 1157 N. W. 11th Ave. Portland, OR 97209 503/225-9181 FAX 503/225-0331

Mascord COLLECTION

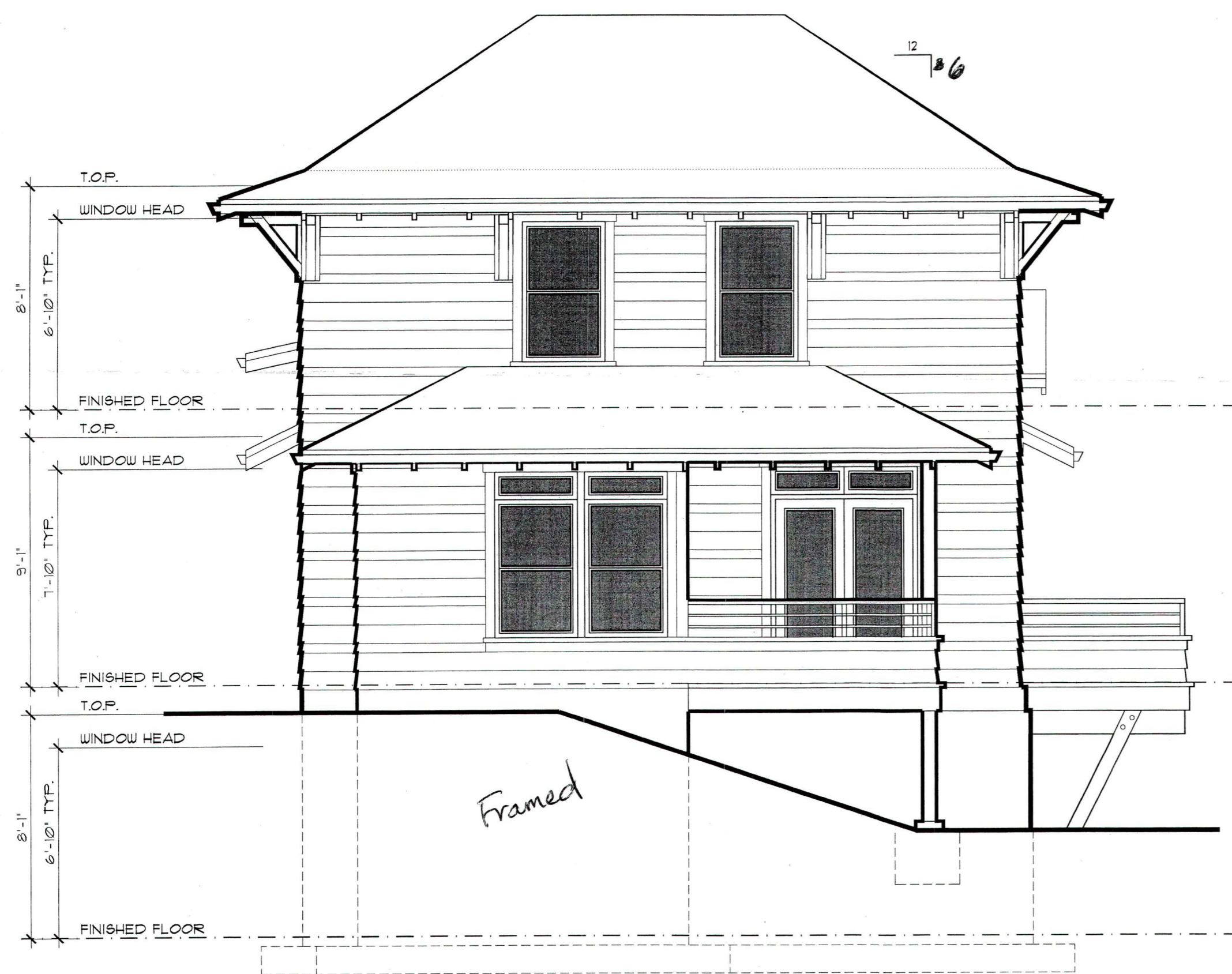
10/4/08
DRAWN 12/01/07 AMP

UPPER FLOOR	400 SQ. FT.
MAIN FLOOR	1352 SQ. FT.
LOWER FLOOR	538 SQ. FT.
TOTAL AREA	2300 SQ. FT.

Halliburton

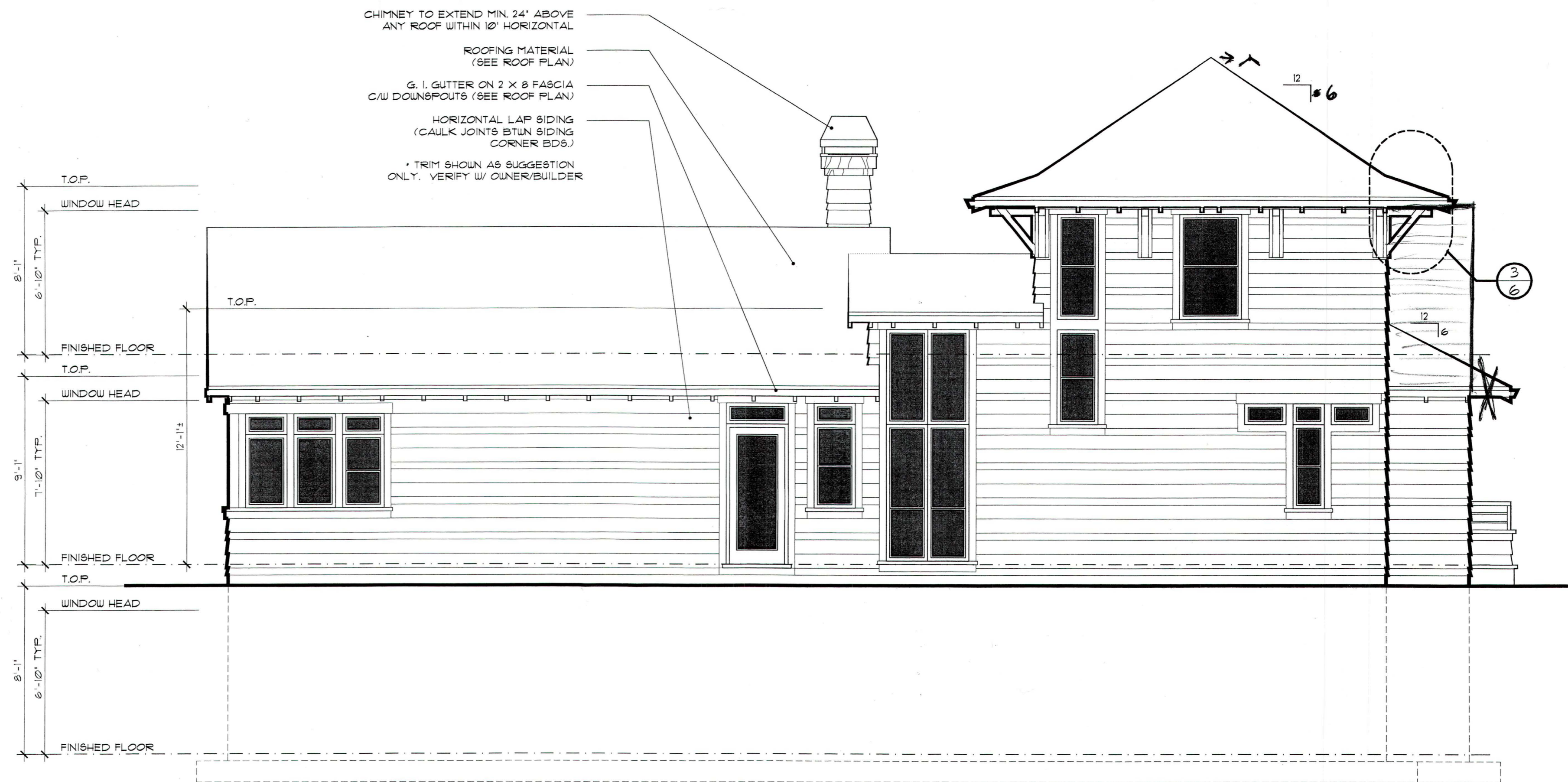
22104
1

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LEFT ELEVATION

SCALE: 1/4" = 1'-0"



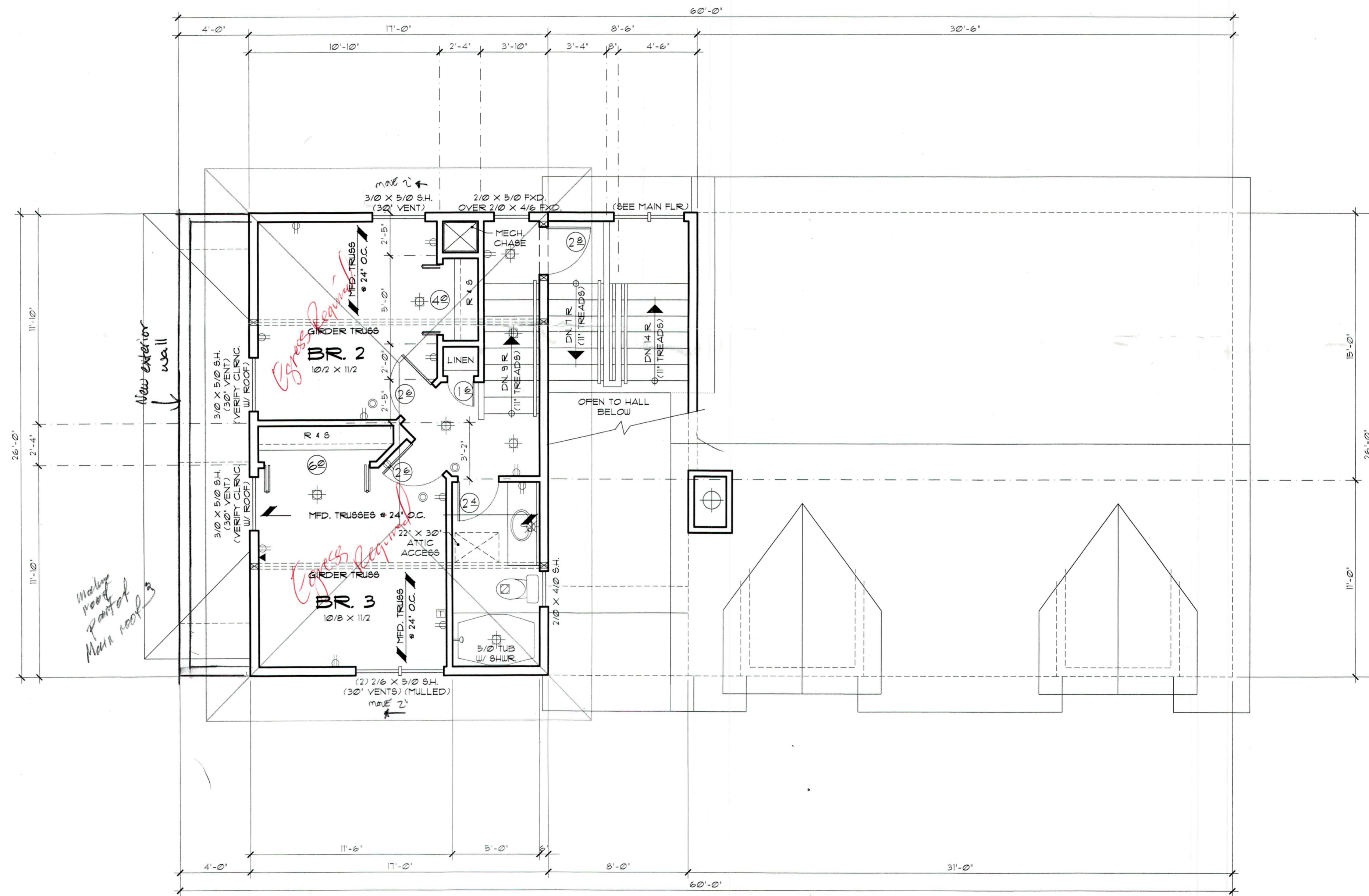
REAR ELEVATION

SCALE: 1/4" = 1'-0"

22104-2
 DRAWN 12/01/97 AMP

UPPER FLOOR	400 SQ. FT.
MAIN FLOOR	1382 SQ. FT.
LOWER FLOOR	538 SQ. FT.
TOTAL AREA	2300 SQ. FT.

22104
 2



- ### LEGEND
- ⊕ RECESSED INCANDESCENT
 - ⊕ RECESSED DIRECTIONAL INCANDESCENT FIXTURE
 - ⊗ WALL MOUNTED INCANDESCENT
 - ⊕ SURFACE MOUNTED INCANDESCENT
 - SURFACE MOUNTED FLUORESCENT
 - ⊗ RECESSED EXHAUST FAN VENTED TO THE EXTERIOR
 - ⊗ 'CASABLANCA' TYPE CEILING FAN
 - ⊕ DUPLEX OUTLET
 - ⊕ CEILING MOUNTED DUPLEX OUTLET
 - ⊕ 220V OUTLET
 - ⊕ FLUSH FLOOR MOUNTED OUTLET (VERIFY LOC.)
 - ▲ TELEPHONE OUTLET
 - ⊕ TELEVISION OUTLET
 - ⊕ SPEAKER LOCATION
 - ⊕ 110 VOLT SMOKE DETECTOR (SEE 'GENERAL NOTES' FOR OTHER SPEC'S)
 - 4 x 4 POST FROM ROOF HIP, VALLEY OR RIDGE DOWN TO BEARING POINT ON WALL BELOW (MAX. OF 45° FROM VERT.)
 - BEARING WALL SUPPORTING STRUCTURE ABOVE
 - 4 x 12 HDR. OR DR. OPENINGS OR FLUSH FLOOR FRAMING MEMBERS W/ BUILT-UP COL. OF 3'x BM. WIDTH EA. END
 - ⊕ DROPPED STRUCT. MEMBER (PROVIDE COLUMN EACH END - MINIMUM OF WIDTH OF THE BEAM)
- S-RULND 06/23/97

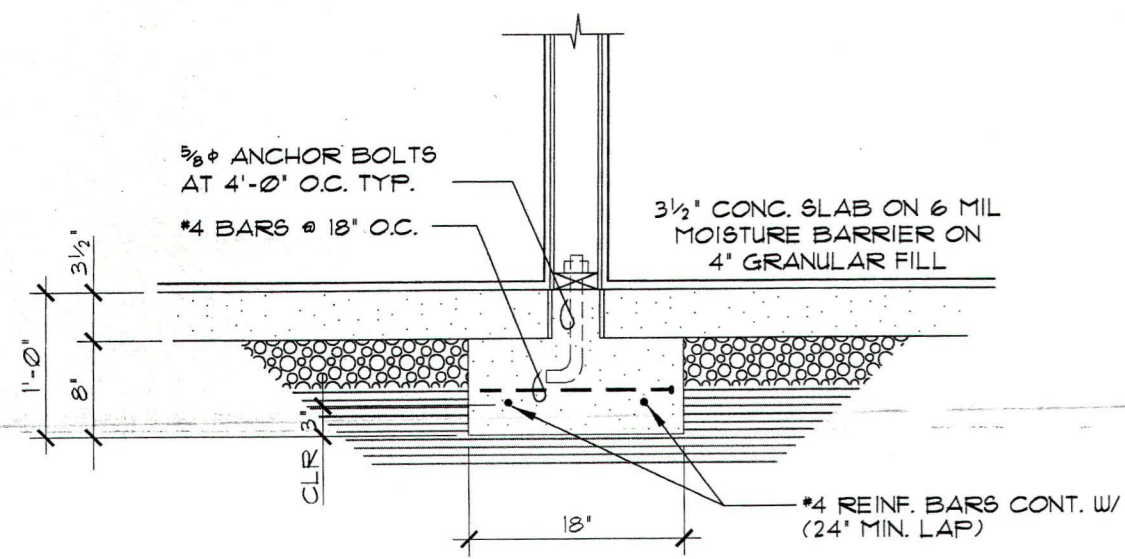
UPPER FLOOR PLAN

SCALE: 1/4" = 1'-0"

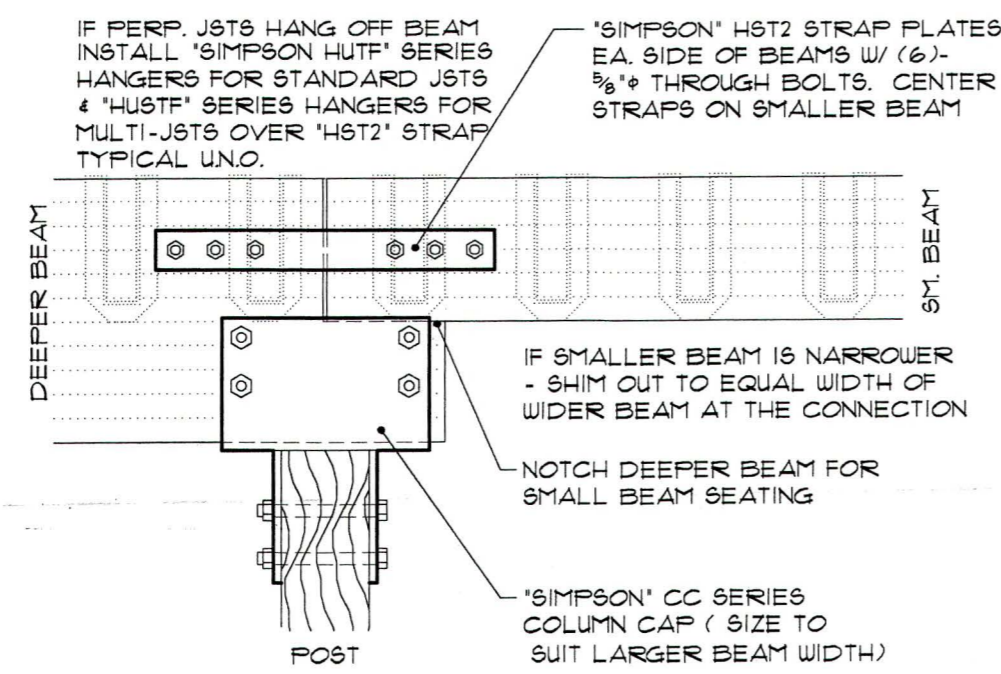
22104-1
DRAWN 12/01/97 AMP

UPPER FLOOR	450 SQ. FT.
MAIN FLOOR	1302 SQ. FT.
LOWER FLOOR	538 SQ. FT.
TOTAL AREA	2300 SQ. FT.

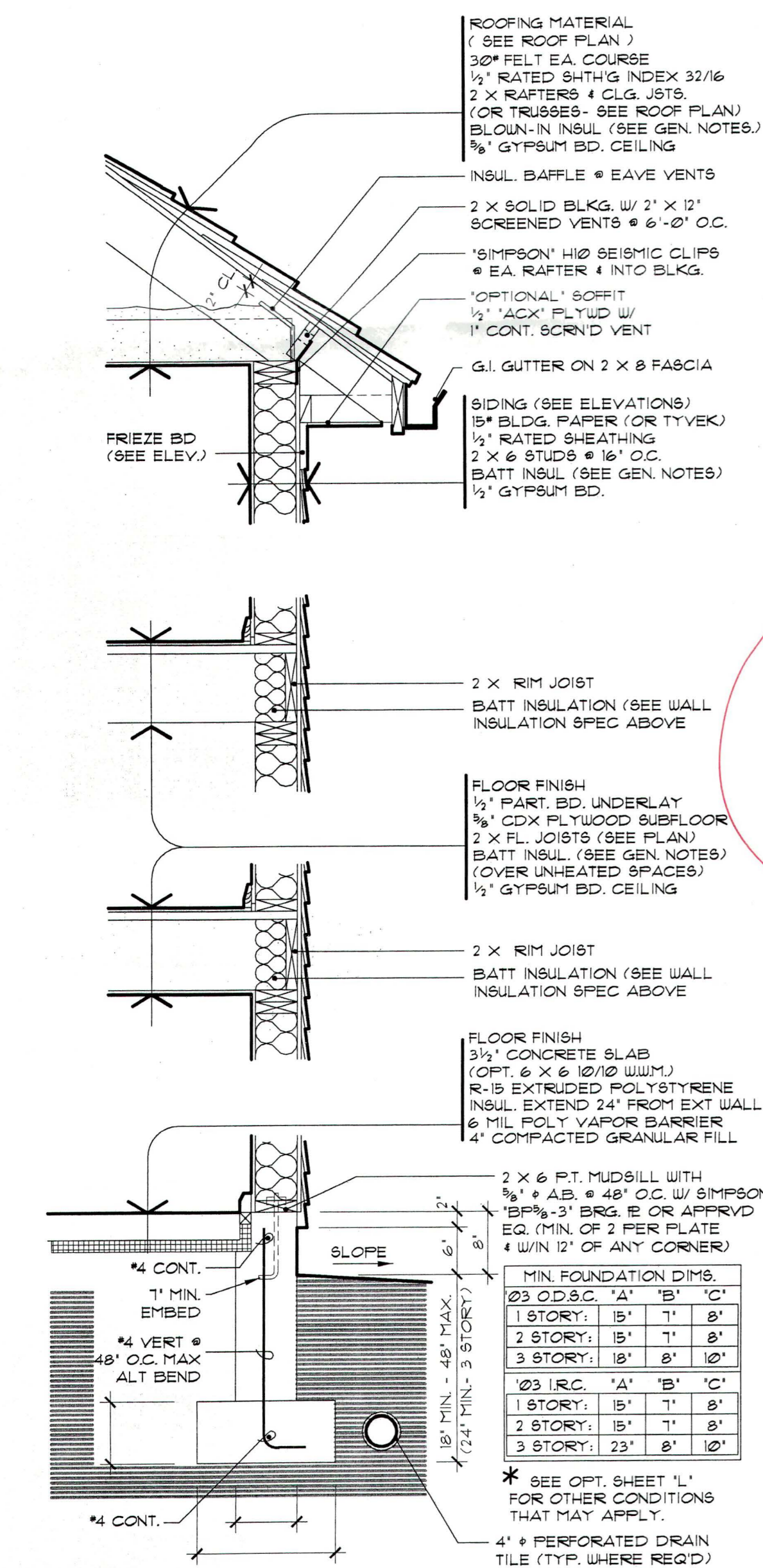
22104
4



1 DETAIL
SCALE: 3/4" = 1'-0"

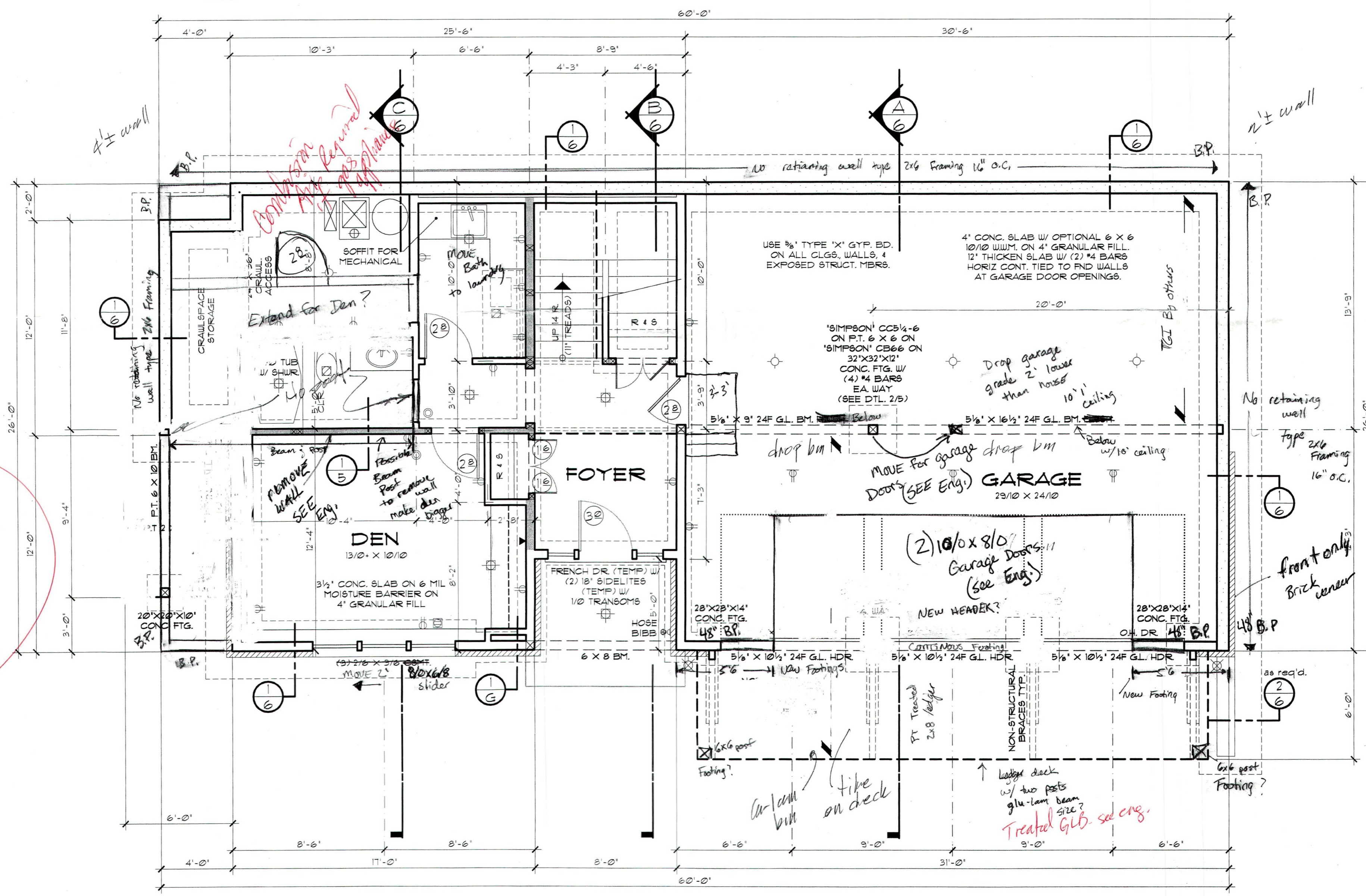


2 BEAM CONNECTION JOINING UNEQUAL BEAMS OVER POST
NO SCALE



TYP. WALL SECTION
SCALE: 3/4" = 1'-0"

*Under slab
Insulation
and Vapor
Barrier
Required*



LOWER FLOOR PLAN
SCALE: 1/4" = 1'-0"

LEGEND

- ⊞ RECESSED INCANDESCENT
- ⊞ RECESSED DIRECTIONAL INCANDESCENT FIXTURE
- ⊞ WALL MOUNTED INCANDESCENT
- ⊞ SURFACE MOUNTED INCANDESCENT
- ⊞ SURFACE MOUNTED FLUORESCENT
- ⊞ RECESSED EXHAUST FAN VENTED TO THE EXTERIOR
- ⊞ 'CASABLANCA' TYPE CEILING FAN
- ⊞ DUPLEX OUTLET
- ⊞ CEILING MOUNTED DUPLEX OUTLET
- ⊞ 220V OUTLET
- ⊞ FLUSH FLOOR MOUNTED OUTLET (VERIFY LOC.)
- ⊞ TELEPHONE OUTLET
- ⊞ TELEVISION OUTLET
- ⊞ SPEAKER LOCATION
- ⊞ 110 VOLT SMOKE DETECTOR (SEE 'GENERAL NOTES' FOR OTHER SPEC'S)
- ⊞ 4 X 4 POST FROM ROOF HIP, VALLEY OR RIDGE DOWN TO BEARING POINT ON WALL BELOW (MAX. OF 45' FROM VERT.)
- ⊞ BEARING WALL SUPPORTING STRUCTURE ABOVE
- ⊞ 4 X 12 HDR. @ DR. OPENINGS OR FLUSH FLOOR FRAMING MEMBERS W/ BUILT-UP COL. OF 3' X 5" MIN. WIDTH EA. END
- ⊞ DROPPED STRUCT. MEMBER (PROVIDE COLUMN EACH END - MINIMUM OF WIDTH OF THE BEAM)

5-FRULND 06/23/97

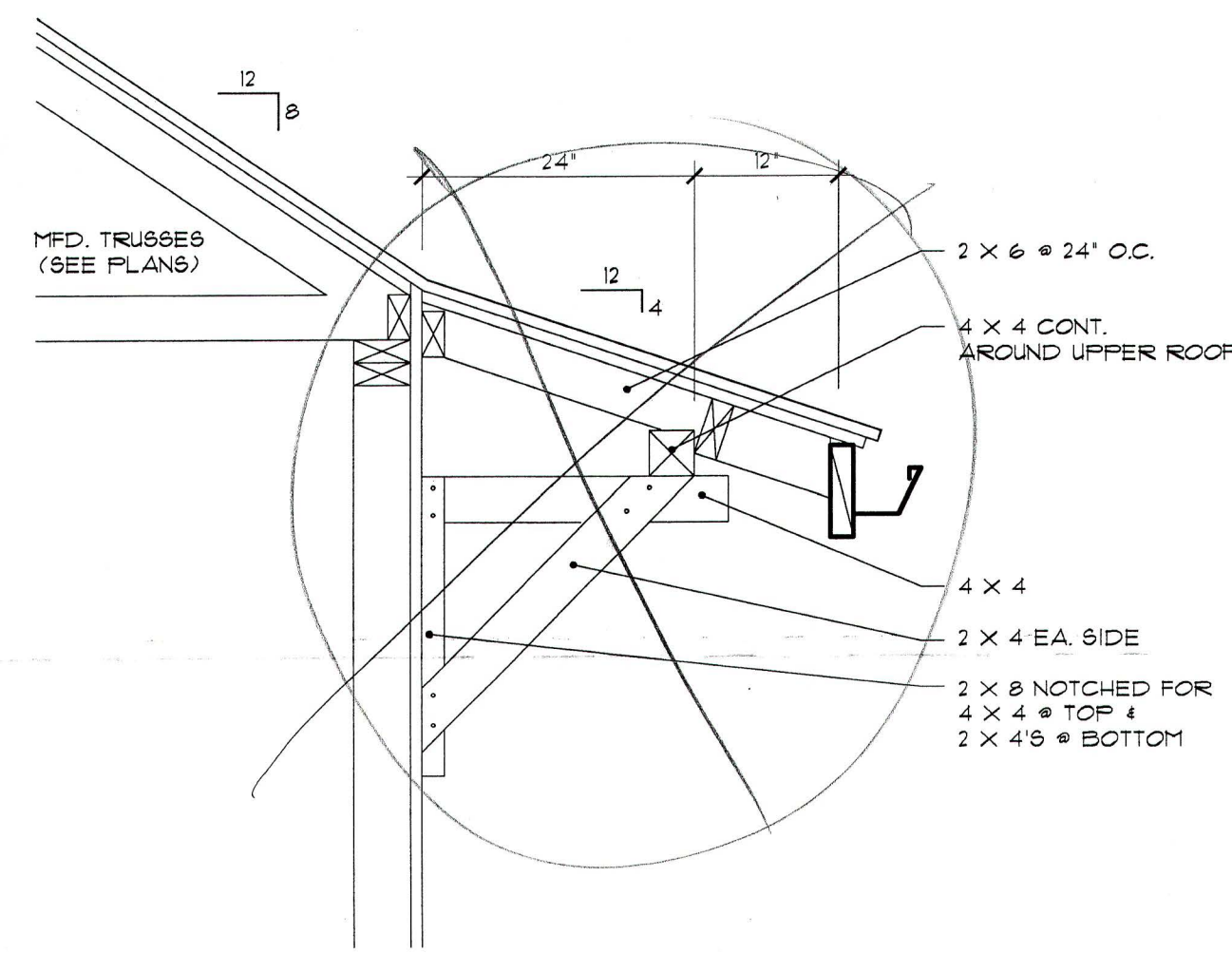
UPPER FLOOR	400 SQ. FT.
LOWER FLOOR	538 SQ. FT.
TOTAL AREA	2300 SQ. FT.

22104
5

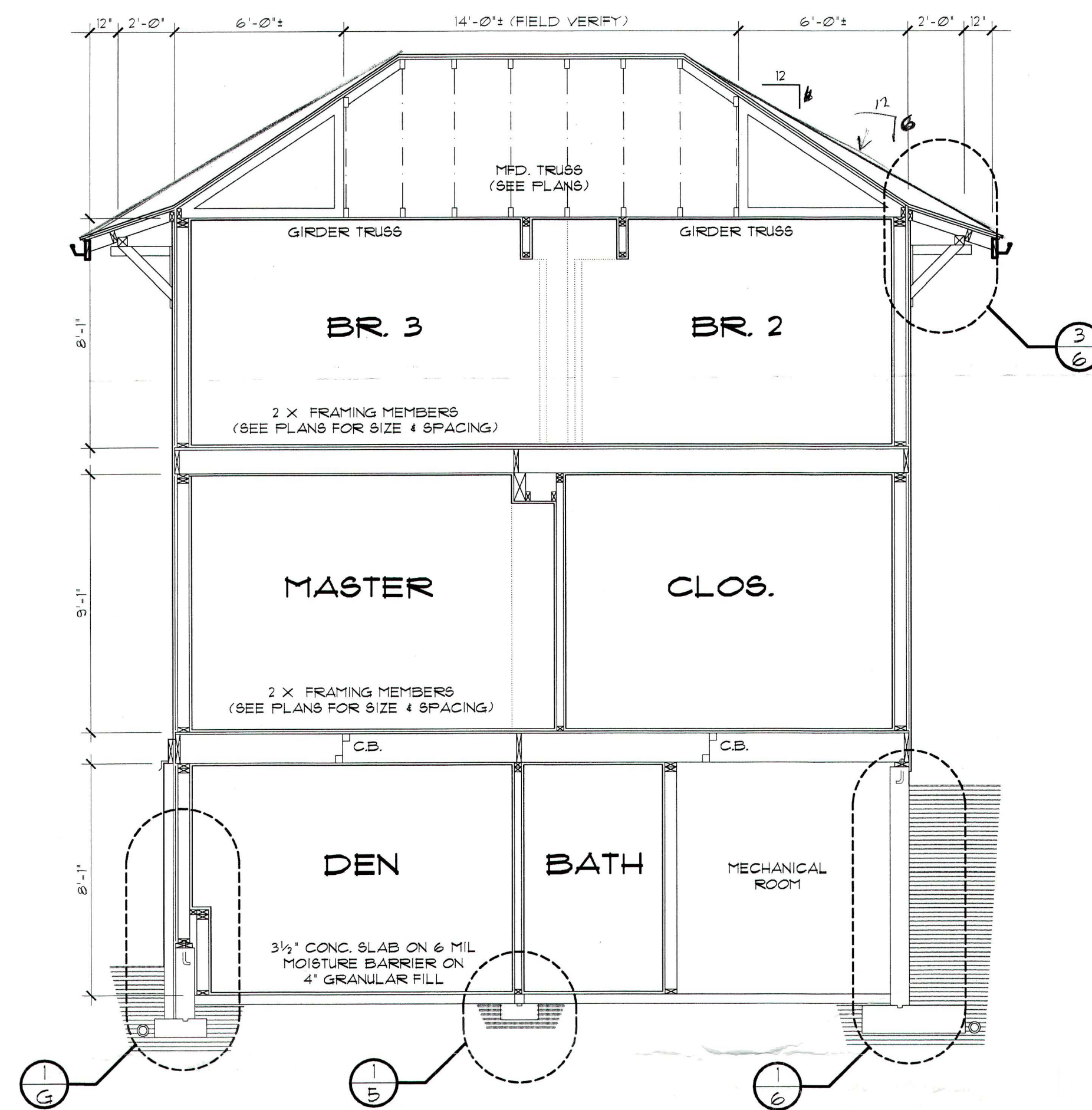
Mascard
COLLECTION

ALAN MASCARD DESIGN ASSOCIATES, INC.
1325 NW 8TH AVE., PORTLAND, OR 97209
TEL: 503/222-9161 FAX: 503/222-0933

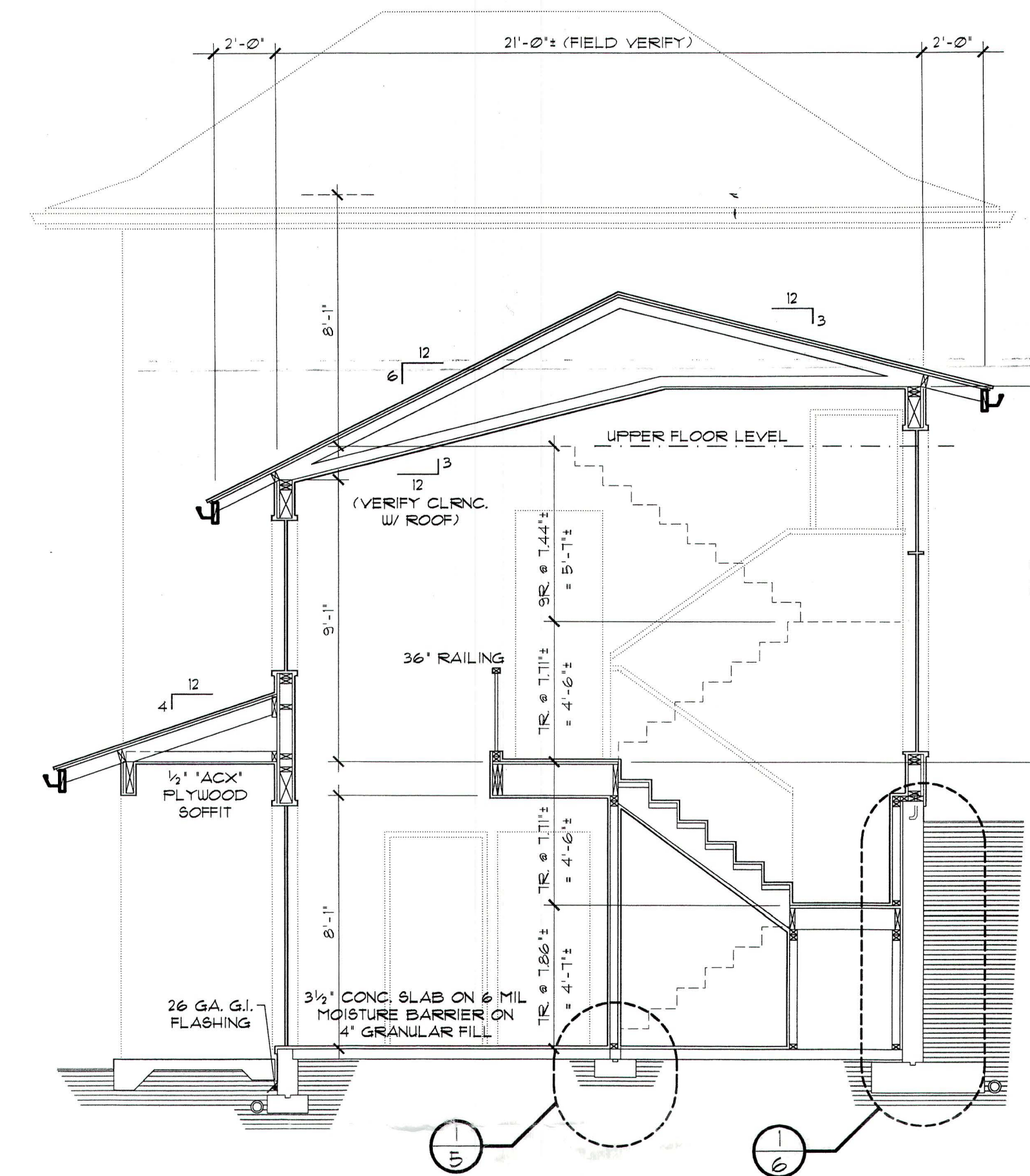
12 2 1 0 4 - 1 2
DRAWN 12/01/97 AMP



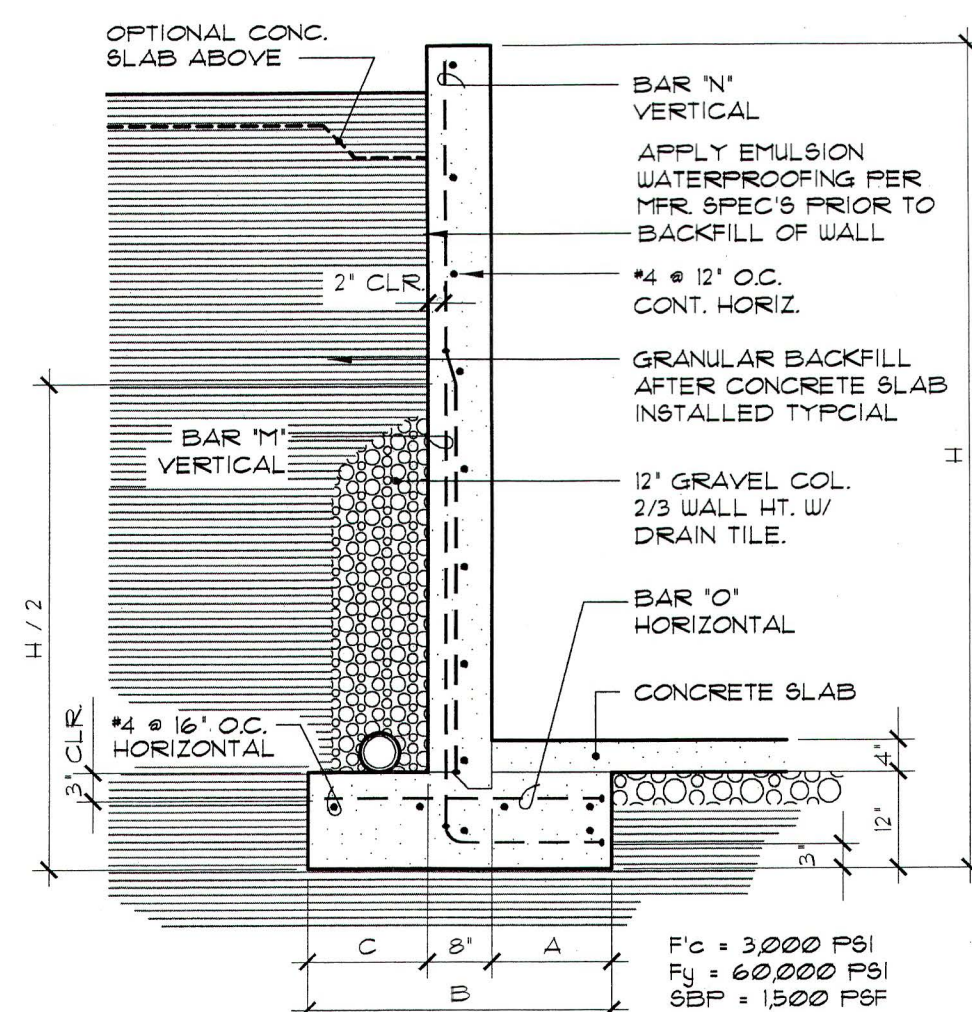
3 EAVE DETAIL
SCALE: 3/4" = 1'-0"



C BUILDING SECTION
SCALE: 1/4" = 1'-0"



B BUILDING SECTION
SCALE: 1/4" = 1'-0"



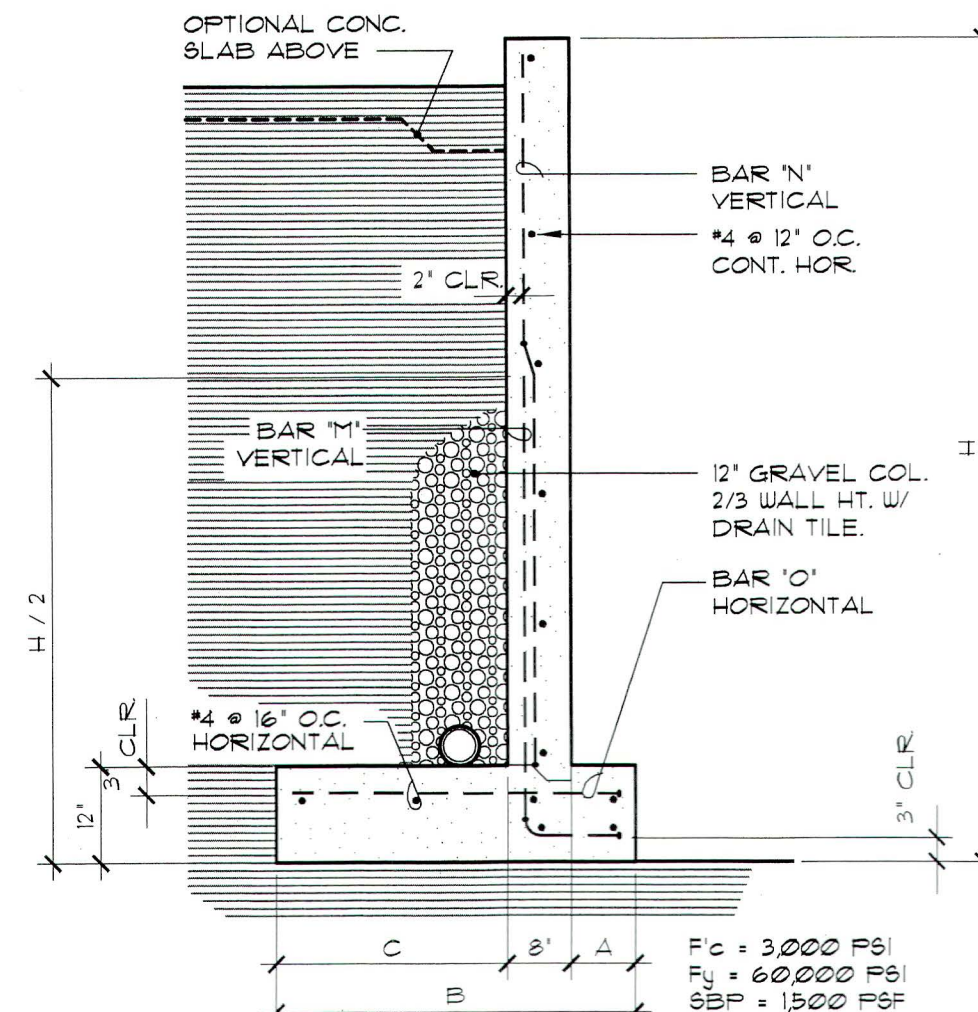
35 PSF EQUIVALENT FLUID PRESSURE RETAINING WALL DESIGN

H	A	C	B	BAR 'M'	BAR 'N'	BAR 'O'
5'-4"	10'	10'	2'-4"	4" @ 18" O.C.	4" @ 18" O.C.	4" @ 18" O.C.
7'-4"	1'-2"	1'-2"	3'-0"	4" @ 18" O.C.	4" @ 18" O.C.	4" @ 18" O.C.
9'-4"	1'-8"	1'-8"	4'-0"	4" @ 18" O.C.	4" @ 18" O.C.	4" @ 18" O.C.
10'-4"	2'-2"	2'-2"	4'-8"	5" @ 9" O.C.	4" @ 18" O.C.	4" @ 18" O.C.

40 PSF EQUIVALENT FLUID PRESSURE RETAINING WALL DESIGN

H	A	C	B	BAR 'M'	BAR 'N'	BAR 'O'
5'-4"	10'	10'	2'-4"	4" @ 18" O.C.	4" @ 18" O.C.	4" @ 18" O.C.
7'-4"	1'-3"	1'-3"	3'-2"	4" @ 18" O.C.	4" @ 18" O.C.	4" @ 18" O.C.
9'-4"	1'-10"	1'-10"	4'-4"	4" @ 18" O.C.	4" @ 18" O.C.	4" @ 18" O.C.
10'-4"	2'-2"	2'-2"	5'-0"	5" @ 9" O.C.	4" @ 18" O.C.	4" @ 18" O.C.

1 RETAINING WALL
SCALE: 1/2" = 1'-0"



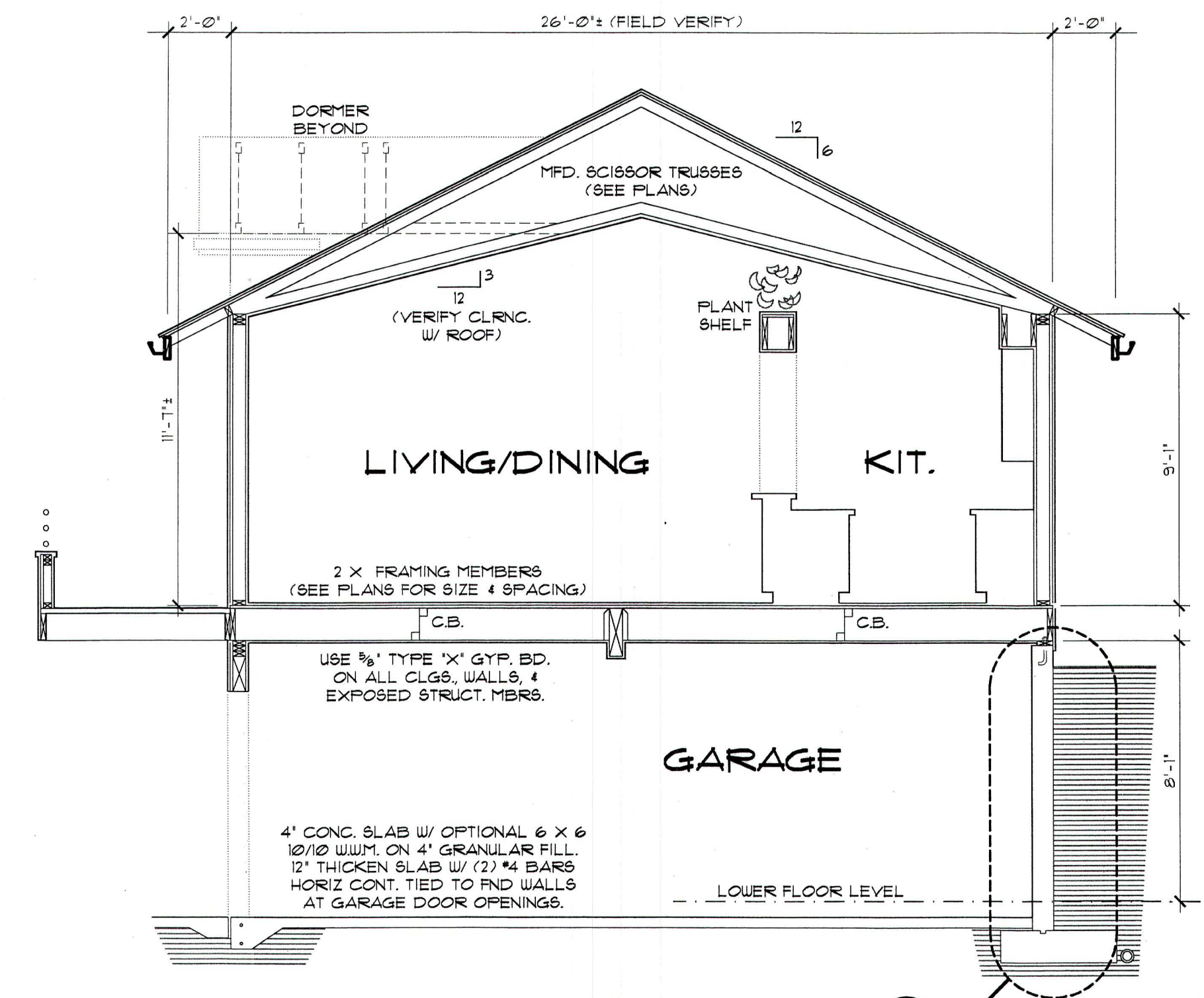
35 PSF EQUIVALENT FLUID PRESSURE RETAINING WALL DESIGN

H	A	C	B	BAR 'M'	BAR 'N'	BAR 'O'
4'-0"	8'	1'-4"	2'-8"	4" @ 18" O.C.	4" @ 18" O.C.	4" @ 18" O.C.
6'-0"	8'	2'-10"	4'-2"	4" @ 18" O.C.	4" @ 18" O.C.	4" @ 18" O.C.
8'-0"	8'	4'-2"	5'-6"	4" @ 16" O.C.	4" @ 16" O.C.	4" @ 12" O.C.
10'-0"	12'	5'-6"	7'-2"	4" @ 6" O.C.	4" @ 18" O.C.	5" @ 8" O.C.

40 PSF EQUIVALENT FLUID PRESSURE RETAINING WALL DESIGN

H	A	C	B	BAR 'M'	BAR 'N'	BAR 'O'
4'-0"	8'	1'-10"	3'-2"	4" @ 18" O.C.	4" @ 18" O.C.	4" @ 18" O.C.
6'-0"	8'	3'-4"	4'-8"	4" @ 18" O.C.	4" @ 18" O.C.	4" @ 18" O.C.
8'-0"	8'	5'-0"	6'-4"	4" @ 12" O.C.	4" @ 12" O.C.	5" @ 12" O.C.
10'-0"	12'	6'-6"	8'-2"	4" @ 6" O.C.	4" @ 18" O.C.	5" @ 8" O.C.

2 RETAINING WALL
SCALE: 1/2" = 1'-0"



A BUILDING SECTION
SCALE: 1/4" = 1'-0"

ROOF DESIGN NOTES

THIS ROOF HAS BEEN DESIGNED TO SUPPORT CEDAR SHAKE ROOFING MATERIALS AND COMPOSITION ROOFING OF VARIOUS TYPES. THE TABLE BELOW DESCRIBES IN DETAIL THE ASSUMPTIONS MADE IN THE DESIGN OF THE ROOF STRUCTURE OF THIS BUILDING.

ROOF LIVE LOAD (SNOW)	25.0 PSF
FRAMING MATERIALS:	2.0 PSF
SHEATHING MATERIALS:	1.5 PSF
MISC. MATERIALS:	1.5 PSF
ROOFING TYPE	DRY / WET
MED SHAKES	2.0 / 3.25 PSF
HVT SHAKES	3.0 / 4.0 PSF
SHINGLES	2.0 / 3.25 PSF
COMPOSITION	2.5 / 3.0 PSF
GYPSUM MATERIALS: ADD 2.0 PSF FOR VAULTED AREAS (COVERED IN SAFETY FACTOR)	32.5 PSF AVE (WET) 6.75 PSF SAFETY FACTOR 40.0 PSF TL

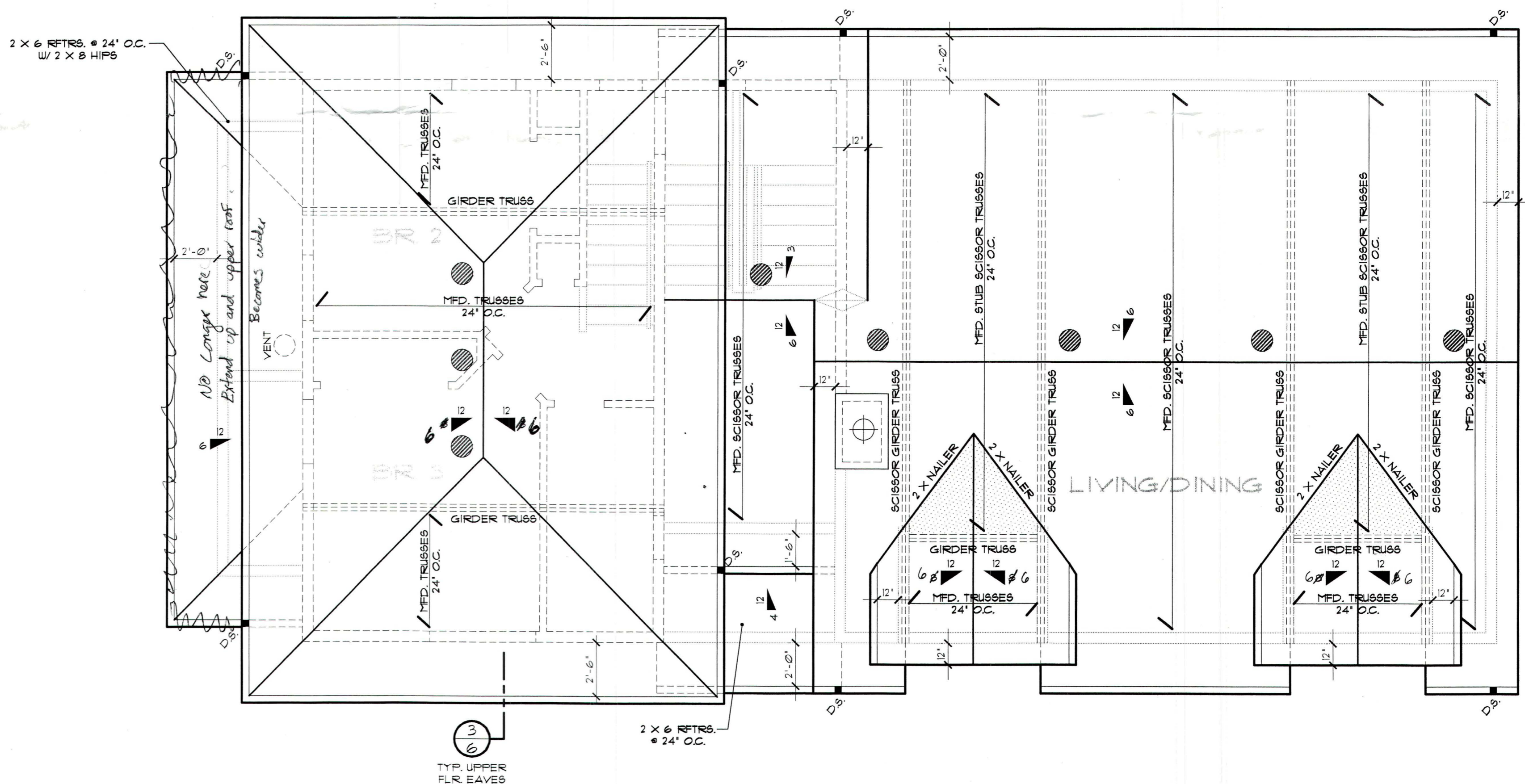
LEGEND

- 4 X 4 WOOD POST FROM RIDGE (HIP OR VALLEY) TO WALL BELOW (MIN. (2) 2 X 4 REQ'D AT WALL BEARING POINT) NOTE: SPLICES IN HIP & VALLEYS CAN ONLY OCCUR @ POST DOWN LOCATIONS
- 49 SQ. IN. ROOF VENTS (3) REQ'D
- 2X4 FURLIN WALL TO BM. OR WALL BELOW (FRAM'G AT 24" O.C.)
- SHADED AREA DENOTES ROOF FRAMED OVER RAFTERS BELOW
- 0" DOWNSOUTS

COMP. OR SHAKE ROOFING

SIZE	SPACING	SPAN
2X6	12' O.C.	13'-5"
	16' O.C.	11'-11"
	24' O.C.	9'-8"
2X8	12' O.C.	17'-2"
	16' O.C.	15'-0"
	24' O.C.	12'-3"
2X10	12' O.C.	21'-2"
	16' O.C.	18'-5"
	24' O.C.	15'-0"
2X12	12' O.C.	24'-8"
	16' O.C.	21'-4"
	24' O.C.	17'-5"

8-30-95 6-LOADS



ROOF PLAN

SCALE: 1/4" = 1'-0"

UPPER FLOOR	480 SQ. FT.
MAIN FLOOR	832 SQ. FT.
LOWER FLOOR	538 SQ. FT.
TOTAL AREA	2300 SQ. FT.

General Notes:

- The structural design depicted on these plans has only been developed to a certain level of completeness (not 100% detailed). Connections not shown shall be determined by the contractor but shall meet the minimum requirements of the code. Basic members and their special connections (if applicable) have been designed as shown. It is the understanding of the engineer that the project will be adjusted as necessary to meet the prescriptive lateral load path designated in the Code. It is the responsibility of the builder and/or contractor to meet all applicable building codes for the relevant municipality. It is imperative that the builder and/or contractor possess competence in residential construction and the knowledge required to install standard structural details and connections.
- The structural system of this building has been partially engineered as allowed by the Governing Building Code. Areas of work not specifically addressed on these plans must meet the "prescriptive" requirements of the Governing Building Code and the requirements of the local building official for conventional construction. The following systems have been engineered by Haffner Consulting Engineering: foundation system for gravity loads, floor framing system for gravity loads, wall components for lateral loads, roof framing system for gravity loads, gravity load connections. The following items are to be designed by others as specified: floor framing system (to include joists), roof framing system (to include trusses), wall bracing system (conventional construction).
- These structural drawings are intended to be used in conjunction with a set of drawings produced by Alan Mascord Design Associates, Inc. and modified by the owner. Refer to others for all dimensions and details not shown on these drawings. The contractor is responsible for coordinating all disciplines, including items to be designed by others, with these drawings.
- Haffner Consulting Engineering shall not have control over, be in charge of and shall not be responsible in any way for construction means, methods, techniques, sequences, or procedures or for job site safety or for safety precautions, requirements or regulations.
- Note that "special inspections" of certain portions of this design may be required by local building officials. Also see Special Inspections section below (if applicable) for special inspections required by the engineer.
- The contractor shall verify all dimensions, elevations, and site conditions before starting work. Any discrepancy, inconsistency, omission and/or conflict between the various elements of the drawings shall be brought to the attention of the engineer immediately before proceeding with any related work. In no case shall dimensions be scaled from plans, sections or details on these drawings.
- Specific notes and details take precedence over "General Notes" and "Typical Details".
- The structural drawings represent the finished structure. They do not indicate the method of construction. The contractor shall provide all measures necessary to properly align and protect the structure during construction. Such measures shall include, but not be limited to: bracing, shoring for loads due to construction equipment, temporary structures, and partially completed work, etc. Observation visits to the site by the engineer shall not include observations of the above items.
- The contractor shall verify all existing conditions, dimensions, and elevations. The contractor shall notify the engineer of any significant discrepancies from the conditions shown on the drawings.
- Governing Building Code: 2007 Edition of the Washington State Building Code
- Roof Truss Design Loading:

Live Load =	52 psf (ground snow load) - Top Chord
	10 psf (no attic storage - not concurrent with other live loads) - Bottom Chord
Dead Load =	15 psf - Top Chord
	10 psf - Bottom Chord
- Floor Design Loading:

Live Load =	40 psf
Dead Load =	10 psf
	20 psf (hinsst tile areas)

General Framing Notes:

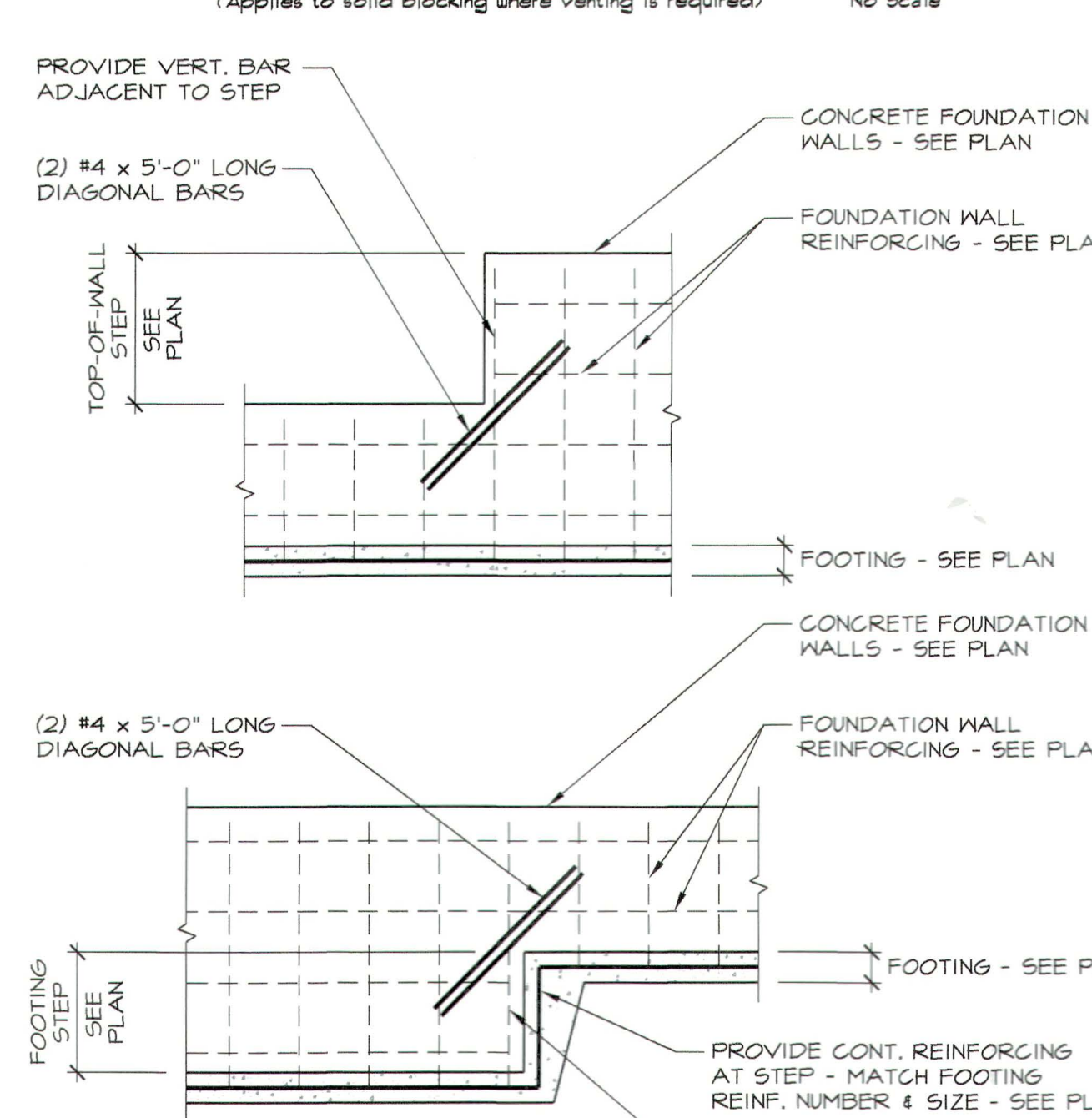
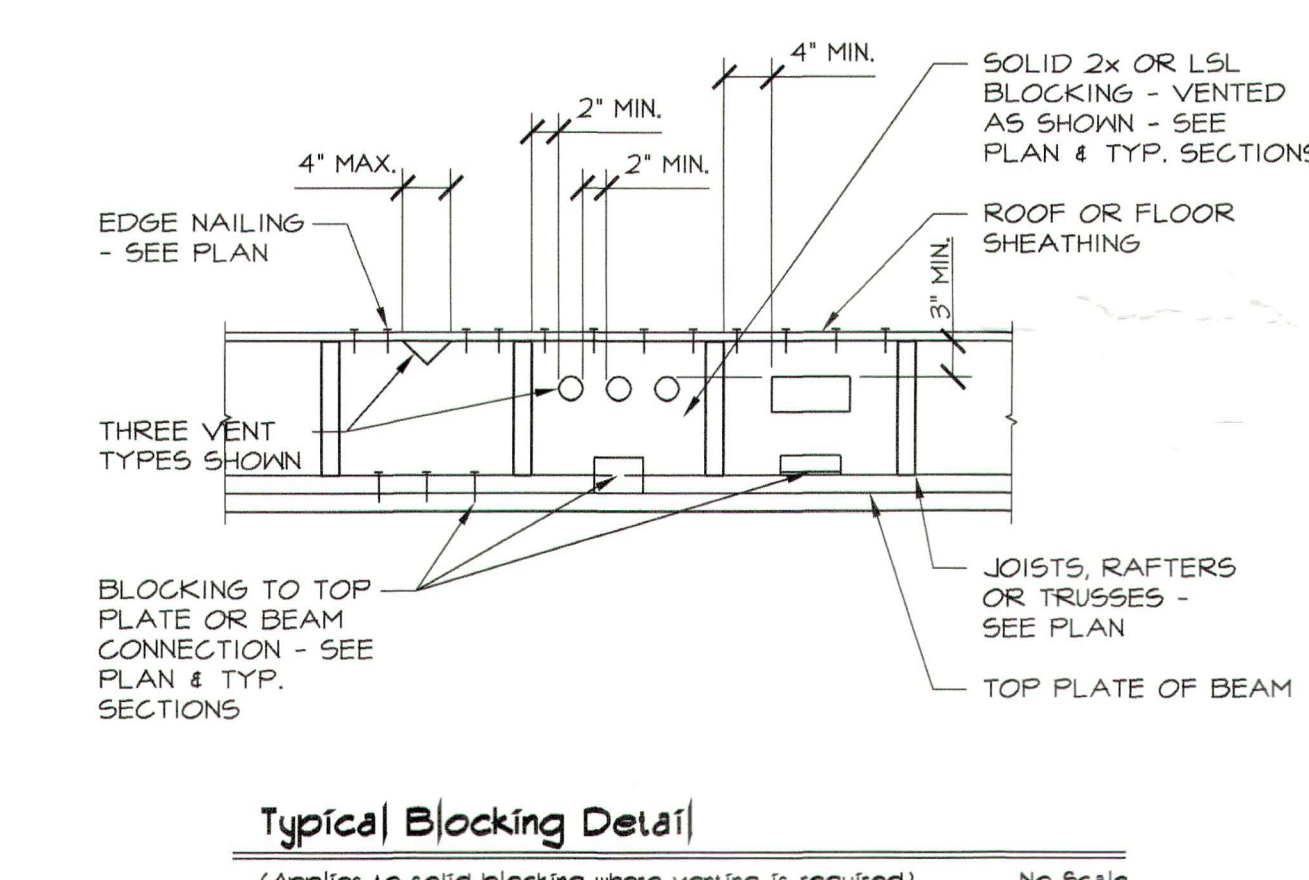
- All structural lumber shall be Douglas Fir-Larch #2 as graded by WWPA or better unless otherwise noted on the drawings. All lumber shall have a maximum moisture content of 19% at the time of installation.
- All structural lumber in contact with concrete or exposed to weather shall be Pressure Treated (PT) Hem-Fir #2 as graded by WWPA or better unless otherwise noted on the drawings.
- Floor joists shall be factory manufactured I-Joists by Boise Cascade Corporation or approved equal. Install joists in strict accordance with manufacturer recommendations.
- All roof trusses, floor joists and beams shall be designed by others in accordance with the loadings specified above and with the Governing Building Code.
- All beam sizes specified are minimums and may be changed to a larger size as necessary for aesthetic purposes.
- All roof trusses, temporary bracing, permanent bracing and truss to truss connections shall be designed and specified by the truss manufacturer. The truss manufacturer under the direction of a licensed professional engineer shall design all of these items for the loads specified and in accordance with the Truss Plate Institute. Unbalanced snow loads shall also be considered as required. Truss shop drawings (including layout) that are sealed by a professional engineer registered in the state in Washington shall be submitted to the architect, owner or contractor for approval prior to truss fabrication. All standard Truss Plate Institute details shall be used for the attachment of non-load-bearing stud walls to both roof and floor trusses.
- Engineered lumber members marked "PSL" (parallel strand lumber) shall have a minimum allowable bending stress (Fb) of 2900 psi, a minimum allowable shear stress (Fv) of 290 psi, and a minimum modulus of elasticity (E) of 2.0×10^6 psi. Engineered lumber members marked "LVL" (laminated veneer lumber) shall have a minimum allowable bending stress (Fb) of 2600 psi, a minimum allowable shear stress (Fv) of 285 psi, and a minimum modulus of elasticity (E) of 1.9×10^6 psi. Engineered lumber members marked "LSL" shall have a minimum allowable bending stress (Fb) of 1700 psi and a minimum modulus of elasticity (E) of 1.3×10^6 psi. All manufacturers' recommendations on nailing and connections shall be followed.
- Glue-Laminated members marked Glu-Lam shall be manufactured as recommended by the American Institute of Timber Construction (AITC). All beams shall be manufactured as required to achieve a Combination Symbol of 24F-V4 using Douglas Fir lumber. The minimum allowable bending stress (Fb) shall be 2400 psi in both the tension and compression zones, the minimum allowable shear stress (Fv) shall be 190 psi, and the minimum modulus of elasticity (E) shall be 1.8×10^6 psi. Glue-Laminated members marked Big Beam shall be manufactured by Rosboro. The minimum allowable bending stress (Fb) shall be 3000 psi in both the tension and compression zones, the minimum allowable shear stress (Fv) shall be 300 psi, and the minimum modulus of elasticity (E) shall be 2.1×10^6 psi.
- A double floor joist shall be installed below all walls that run parallel to the joists.
- Solid blocking between floor joists shall be installed at beam and header locations and below point loads. All solid blocking and rim joist material shall be 1-1/4" thick LSL material (minimum).
- All nailed connections, unless otherwise shown on the drawings, shall be as prescribed by the Building Code and any associated nailing shall be as prescribed by the Governing Building Code in Table R602.3(1) or Table 2304.9.1.
- All nails for specified connections shall be common wire nails (unless noted otherwise) with the following minimum sizes. Alternate nails shall not be used without prior written approval of the Engineer.

Common nails	
8d	0.131" diameter x 2 1/2" long
10d	0.148" diameter x 3" long
16d	0.162" diameter x 3 1/2" long
IF SPECIFICALLY SPECIFIED TO BE USED nails shall be the following minimum sizes.	
Sinker nails	
8d	0.113" diameter x 2-3/8" long
10d	0.120" diameter x 2-7/8" long
16d	0.148" diameter x 3-1/4" long
Box nails	
8d	0.113" diameter x 2-1/2" long
10d	0.128" diameter x 3" long
16d	0.135" diameter x 3-1/2" long
- ALL anchor bolts, nails and other connectors in contact with corrosive pressure treated wood shall be hot dipped galvanized or stainless steel. This includes shearwall sheathing nailing and any other nails or connectors that contact pressure treated sill plates, ledgers or other treated members.
- All wood framing connectors shall be by Simpson or approved equal unless noted otherwise on the drawings. Install connectors in strict accordance with manufacturer's recommendations.
- All anchor bolts shall be ASTM A307 or F1554 and shall have a minimum yield stress of 36 ksi.
- All structural steel shall conform to the following:

Structural Steel (W & S Shapes)	ASTM A992
Structural Steel	ASTM A36
Miscellaneous Steel	ASTM A36
Structural Tubing (HSS shapes)	ASTM A500, Grade B
Steel Pipe	ASTM A53
- Unless otherwise noted on the drawings, all custom wood-to-wood or wood-to-steel framing connectors shall be designed and specified by a registered professional engineer for the loads shown.
- All welding shall be performed by certified welders. Welding shall conform to the latest publication of applicable codes set forth by the American Welding Society. No unauthorized welds will be accepted.
- All floor and ceiling joists that butt into the side of a header or steel beam shall be anchored to the beam or header with standard joist hangers (Simpson LUS, JB, IUS, or ITS or approved equal).
- All stand-alone wood columns shall be anchored to the framing above with a Simpson PC or EPC standard post cap, unless noted otherwise.
- All stand-alone wood columns shall be anchored to the concrete below with a Simpson ABE or ABU base, unless noted otherwise.

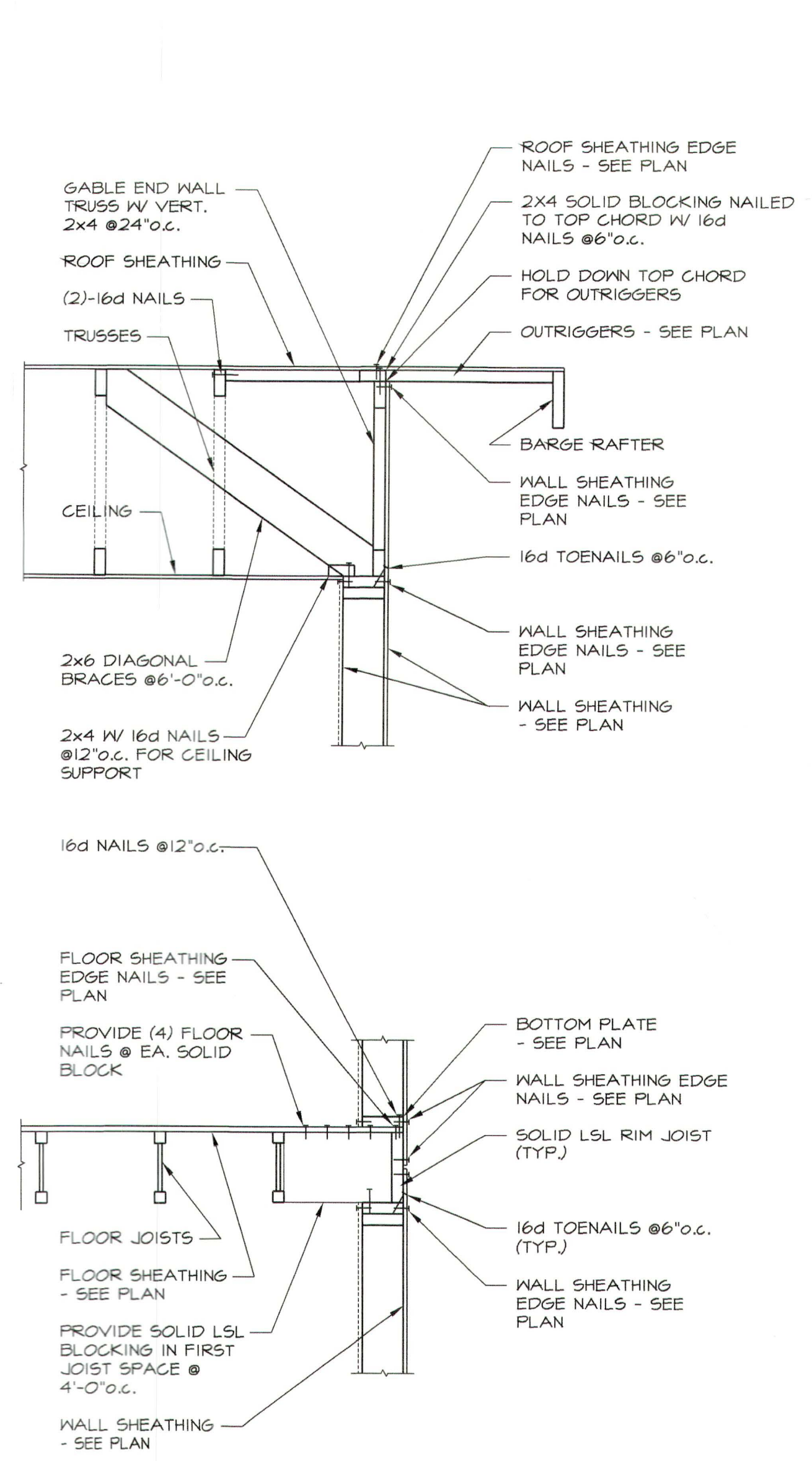
General Foundation Notes:

- Foundations for this project have been designed for an assumed allowable soil bearing value of 1500 psf for continuous and individual footings as allowed by the Building Code. It shall be the responsibility of the contractor to verify that the soil existing at the footing bearing elevation is either clay, sandy clay, silty clay, clayey silt, silt or sandy silt. If upon excavation, it is found that the soil condition is not as assumed it shall be the responsibility of the contractor to notify the engineer so that the foundation design may be altered for the soil conditions encountered at the site.
- If the existing site topography or soil conditions vary from the conditions shown on the drawings, it shall be the responsibility of the contractor to notify the engineer so that a design that is appropriate for the site can be produced.
- The concrete for the footings and foundation walls shall have a minimum 28-day strength of 3000 psi with a maximum slump of 4". The concrete for the floor slabs shall be by others.
- All admixtures shall be approved by the engineer. No water shall be added at the project site without the engineer's permission.
- The reinforcing steel shall be ASTM A615 with the following Grade. All bars shall be lapped a minimum of 24 inches and corner bars shall be provided at all footing and wall corners, and footing steps.
 - Grade 40 for footings, foundation walls and non-structural slabs unless otherwise noted.
 - Grade 60 for all structural slabs unless otherwise noted.
 - Grade 60 for all retaining walls unless otherwise noted.
- All foundation and retaining walls have been designed for an equivalent fluid pressure of 35 pounds per square foot per foot of depth. This requires that a "free draining" backfill be installed.
- Unless otherwise noted on the drawings, all clearance to any reinforcing shall be as follows:
 - concrete cast against earth 3"
 - concrete exposed to earth or weather 1 1/2"
 - concrete not exposed to weather or in contact with ground 3/4"
- Epoxy, if used or specified, shall be Simpson AT (Acrylic Tie), Simpson SET or Simpson ET or approved equal. All manufacturers' recommendations shall be strictly followed.
- Footings shall be poured continuous at footing steps (solid jumps).
- Provide two #4 x 4'-0" long diagonal bars at the bottom two corners of basement windows and crawl space vents. Also provide a horizontal bar below the opening by bending a continuous horizontal bar or providing an additional bar that extends 2'-0" on each side of the opening.
- Any fill that is installed under the basement or garage floor slabs shall be properly compacted to prevent settlement of the fill material.
- Control joints in the floor slabs shall be installed as to minimize the amount of random cracking (20' intervals maximum). These joints shall be sawcut within 12 hours of pouring the slab or may be tooled into the slab when poured.

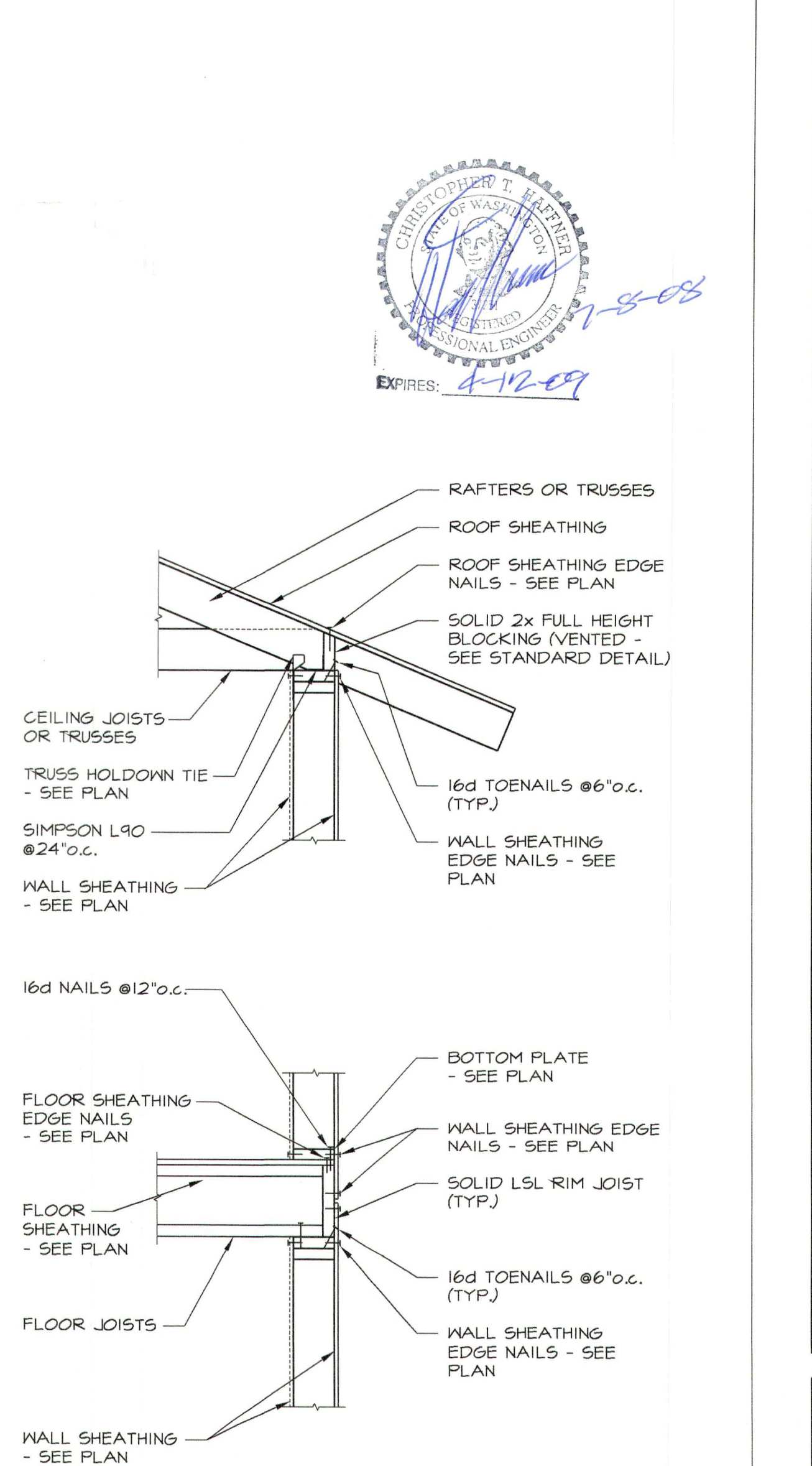


Notes
1. Top-of-wall step shall be more than 4'-0" from footing step.

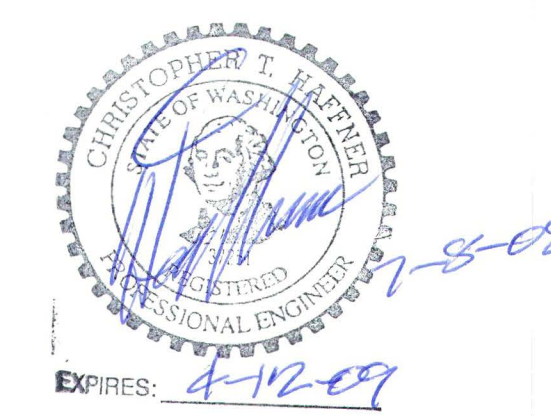
Foundation Steps Standard Detail
No Scale



Typical Wall Section - Exterior Parallel
(Applies to exterior walls parallel to framing) No Scale



Typical Wall Section - Exterior Perpendicular
(Applies to exterior walls perpendicular to framing) No Scale



Hallyburton Residence

Edgecliff Drive
White Salmon, Washington

PROJECT NO.:
2008-115

ISSUE DATE:
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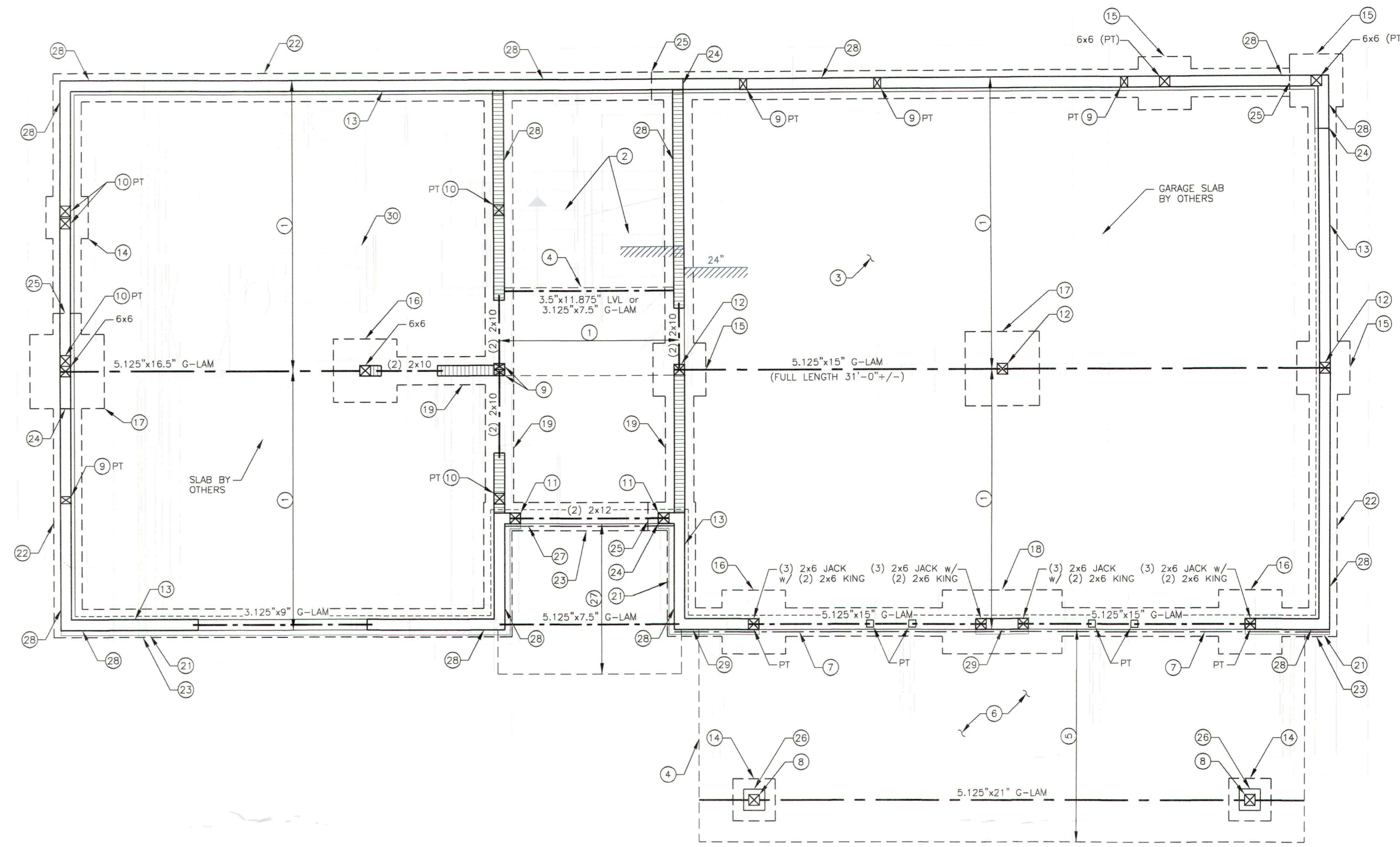
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General Notes & Typical Details
S0.0

Lower Level and Foundation Plan Notes:

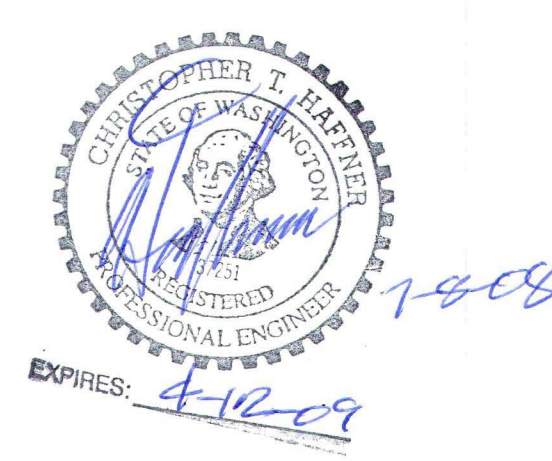
1. 11 7/8" floor joists at 16" o.c. by others.
2. Stair framing by others.
3. Provide 3/4" APA rated floor sheathing (OSB or plywood). Install sheathing in a staggered pattern perpendicular to supporting members. Nail sheathing to all joists with 8d nails at 6" o.c. (edges) and 12" o.c. (field).
4. Floor or deck line above (typical).
5. 2x8 deck joists at 16" o.c.
6. Provide 3/4" APA rated deck sheathing (OSB or plywood). Deck shall be waterproof. Install sheathing in a staggered pattern perpendicular to supporting members. Nail sheathing to all joists with 8d nails at 6" o.c. (edges) and 12" o.c. (field).
7. 2x8 pressure treated deck ledger with 1/2" diameter bolts through rim joist at 16" o.c. Provide LUS joist hangers to deck joists.
8. 6x6 deck column with Simpson CQC or ECCQ cap and Simpson CBSQ or CBQ base.
9. Built-up (2)-2x6 column. Nail 2x studs together with 16d sinkers @12" o.c.
10. Built-up (3)-2x6 column. Nail 2x studs together with 16d sinkers @12" o.c.
11. Balloon frame this wall and provide (1)-2x6 jack stud at headers with (3)-2x6 king studs.
12. 6x6 column with Simpson CQC or ECCQ cap and Simpson CBSQ or CBQ base.
13. All exterior walls to be 2x6 studs @16" o.c. unless noted.
14. 24"x24"x8" concrete footing reinforced with (3) #4's each way. Center below point load above.
15. 30"x30"x8" concrete footing reinforced with (4) #4's each way. Center below point load above.
16. 36"x36"x12" concrete footing reinforced with (5) #4's each way. Center below point load above.
17. 42"x42"x12" concrete footing reinforced with (6) #4's each way. Center below point load above.
18. 36"x68"x12" concrete footing reinforced with #4's at 8" o.c. each way.
19. 8" thick x 16" wide concrete footing reinforced with (2)-#4's, continuous.
20. Interior load-bearing wall (hatched).
21. Brick veneer to be built-up from top-of-footing.
22. 8" thick x 2'-0" tall concrete foundation wall on 8" thick x 16" wide concrete footing reinforced with (2)-#4's, continuous. Foundation wall shall be reinforced with (3)-#4's continuous, horizontal and with #4's at 24" o.c. vertical dowels with 6" hook in footing. Place reinforcing in the center of the wall. Anchor bolts (5/8" diameter) shall be located at 3'-0" o.c. at the top of the wall in the middle of the sill plate except as noted.
23. Below masonry veneer install an 8" thick x 2'-0" tall concrete foundation wall on 8" thick x 20" wide concrete footing reinforced with (2)-#4's, continuous. Center Foundation wall shall be reinforced with (3)-#4's continuous, horizontal and with #4's at 24" o.c. vertical dowels with 6" hook in footing. Place reinforcing in the center of the wall. Anchor bolts (5/8" diameter) shall be located at 3'-0" o.c. at the top of the wall in the middle of the sill plate except as noted.
24. Step top of wall as grade requires. See Typical Detail.
25. Step footing as grade requires. See Typical Detail.
26. 12"x12" concrete column reinforced with (4)-#4's vertical.
27. Pre-engineered wood trusses at 24" o.c. to 2x8 ledger that is attached to each supporting stud with (2)-Simpson 1/4"x3" SDS screws. Provide Simpson H2.5T to top of beam and ledger. See others for truss shape and profile. See General Notes for design loading requirements.
28. Braced wall Panel: Install 48" wide (minimum) braced wall in accordance with Section R602.10.3 of the Governing Building Code.
29. Alternate Braced wall Panel: Install 32" wide (minimum) alternate braced wall panel in accordance with Section R602.10.6 of the Governing Building Code.
30. Backgrounds are provided for informational purposes only. Refer to others for all dimensions and non-structural information.

PT =point load from above. Provide solid blocking as required to transfer load to members below.



Foundation Plan

1/4" = 1'-0"



Hallyburton Residence

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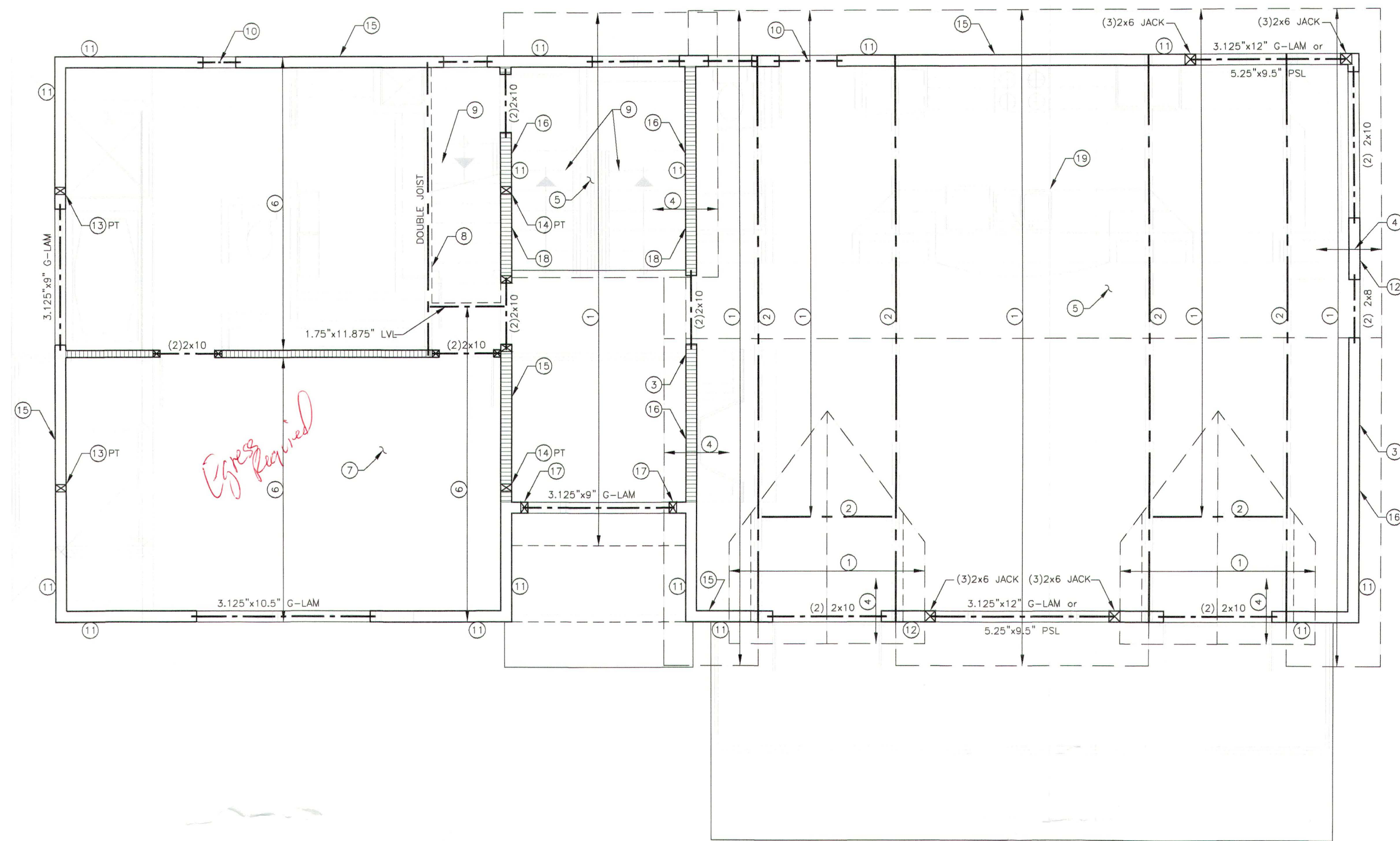
Foundation Plan

S1.0

Main Level Plan Notes:

1. Pre-engineered wood trusses at 24" o.c. Provide Simpson H2.5T to top plate at each bearing location. See others for truss shape and profile. See General Notes for design loading requirements.
2. Pre-engineered wood girder truss. All truss to truss connections shall be designed and specified by the truss manufacturer. Provide Simpson LGT2 to top plate and supporting stud at each bearing location. See others for truss shape and profile.
3. Increase depth of gable end wall truss top chord to allow for notching of top chord for outlookers.
4. 2x4 (flat) outlookers at 24" o.c.
5. Provide APA rated roof sheathing (OSB or plywood) with a minimum Panel Span Rating of 32/16 over all roof trusses (including under overframed areas). Install sheathing in a staggered pattern perpendicular to supporting members. Nail sheathing to all rafters and blocking with 8d nails at 6" o.c. (edges) and 12" o.c. (field).
6. 11 7/8" floor joists at 16" o.c. by others.
7. Provide 3/4" APA rated floor sheathing (OSB or plywood) above. Install sheathing in a staggered pattern perpendicular to supporting members. Nail sheathing to all joists with 8d nails at 6" o.c. (edges) and 12" o.c. (field).
8. Floor or deck line above (typical).
9. Stair framing by others.
10. All exterior wall headers shall be (2)-2x10's or 4x10 unless noted.
11. Braced wall Panel: Install 48" wide (minimum) braced wall in accordance with Section R602.10.3 of the Governing Building Code.
12. Alternate Braced wall Panel: Install 32" wide (minimum) alternate braced wall panel in accordance with Section R602.10.6 of the Governing Building Code.
13. Built-up (2)-2x6 column. Nail 2x studs together with 16d sinkers @ 12" o.c.
14. Built-up (3)-2x6 column (balloon frame at stair wall). Nail 2x studs together with 16d sinkers @ 12" o.c.
15. All exterior walls to be 2x6 studs @ 16" o.c. unless noted.
16. Balloon frame this wall.
17. Balloon frame this wall and provide (1)-2x6 jack stud at headers with (3)-2x6 king studs.
18. Interior load-bearing wall (hatched).

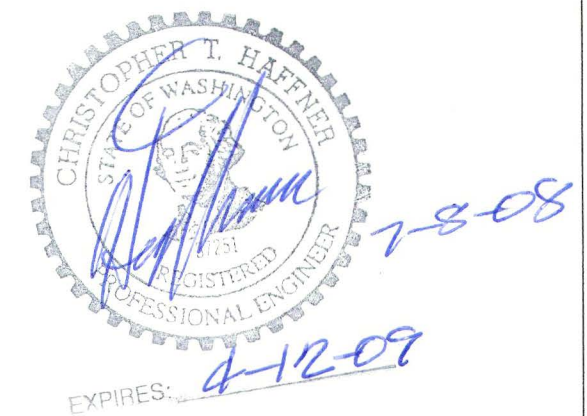
PT = point load from above. Provide solid blocking as required to transfer load to members below.



Egress Required

Main Level Plan

1/4" = 1'-0"



Hallyburton Residence

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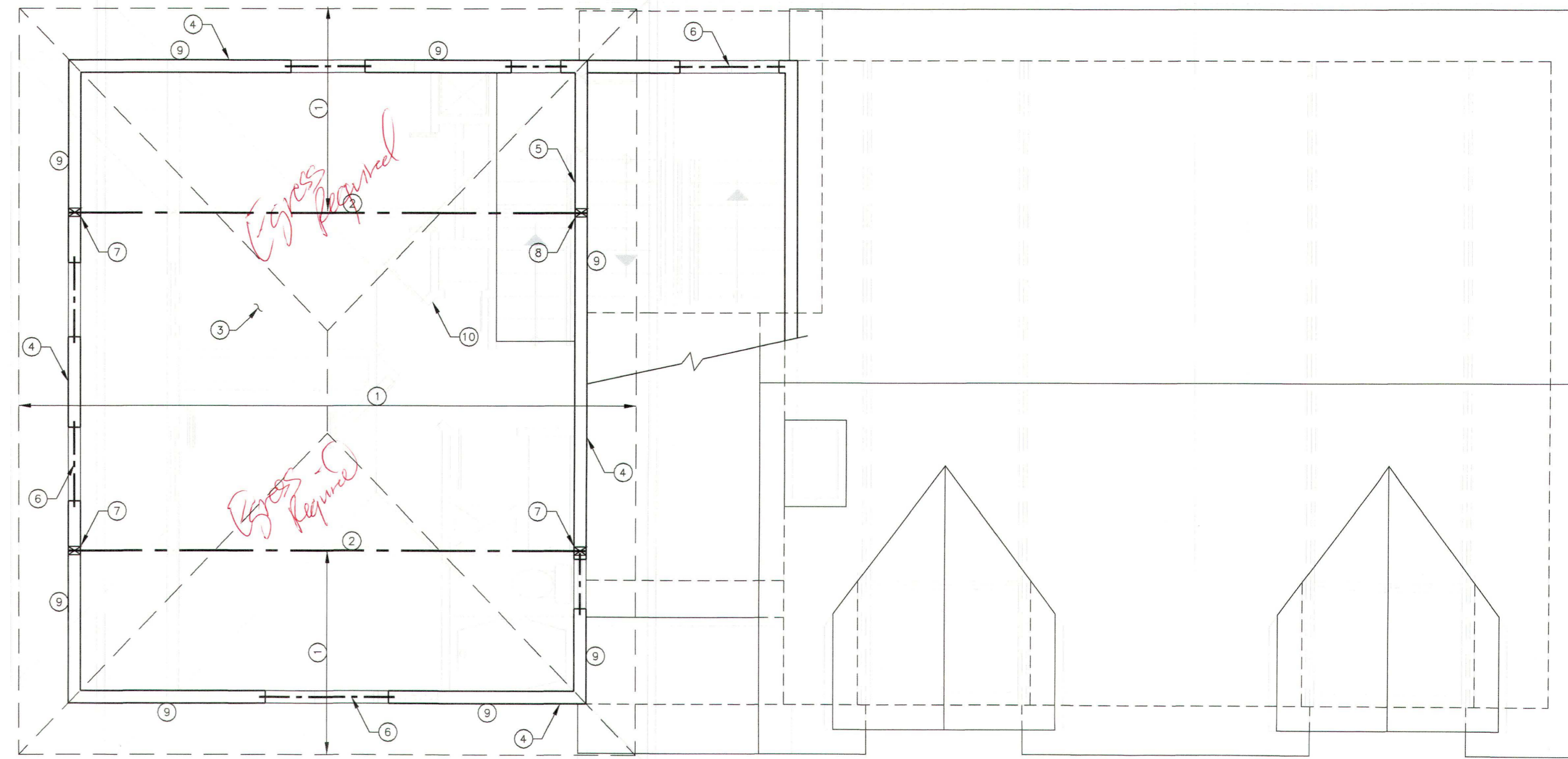
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Main Level
Plan

S2.0

Upper Level Plan Notes:

1. Pre-engineered wood trusses at 24" o.c. Provide Simpson H2.5T to top plate at each bearing location. See others for truss shape and profile. See General Notes for design loading requirements.
2. Pre-engineered wood girder truss. All truss to truss connections shall be designed and specified by the truss manufacturer. Provide Simpson LGT2 to top plate and supporting stud at each bearing location. See others for truss shape and profile.
3. Provide APA rated roof sheathing (OSB or plywood) with a minimum Panel Span Rating of 32/16 over all roof trusses (including under overframed areas). Install sheathing in a staggered pattern perpendicular to supporting members. Nail sheathing to all rafters and blocking with 8d nails at 6" o.c. (edges) and 12" o.c. (field).
4. All exterior walls to be 2x6 studs @16" o.c. unless noted.
5. Balloon frame this stair wall.
6. All exterior wall headers shall be (2)-2x10's or 4x10 unless noted.
7. Built-up (2)-2x6 column. Nail 2x studs together with 16d sinkers @12" o.c.
8. Built-up (3)-2x6 balloon-framed column. Nail 2x studs together with 16d sinkers @12" o.c.
9. Braced wall Panel. Install 48" wide (minimum) braced wall in accordance with Section R602.10.3 of the Governing Building Code.
10. Backgrounds are provided for informational purposes only. Refer to others for all dimensions and non-structural information.



Upper Level Plan

1/4" = 1'-0"



EXPIRES: 4-12-09

Hallyburton Residence

Edgcliff Drive
White Salmon, Washington

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Upper Level
Plan

S3.0